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# Section III

## DESIGN CHARTS, DESIGN DATA AND FORMULAE FOR AIRCRAFT TUBING

The design data of this section are based on the requirements of the  
ANC-5 Handbook, October 1940 revision

### 1. STANDARD SYMBOLS

The following standard symbols are used in this section. These symbols  
correspond to those used in the ANC-5 Handbook.

<p><b>A</b> — area of cross section, square inches</p> <p><b>a</b> — subscript "allowable"</p> <p><b>B</b> — slenderness ratio factor</p> <p><b>b</b> — subscript "bending"</p> <p><b>C</b> — circumference</p> <p><b>c</b> — fixity coefficient for columns; distance from neutral axis to extreme fiber; subscript "compression"</p> <p><b>cr</b> — subscript "critical"</p> <p><b>D</b> — diameter of tube</p> <p><b>D/t</b> — ratio of diameter to thickness of tube</p> <p><b>E</b> — modulus of elasticity in tension</p> <p><b>F</b> — allowable stress</p> <p><b>f</b> — internal (or calculated) stress</p> <p><b>F<sub>b</sub></b> — allowable bending stress, modulus of rupture in bending</p> <p><b>f<sub>b</sub></b> — internal (or calculated) primary bending stress</p> <p><b>F<sub>c</sub></b> — allowable compressive stress</p> <p><b>f<sub>c</sub></b> — internal (or calculated) compressive stress</p> <p><b>F<sub>cc</sub></b> — allowable crushing or crippling stress (upper limit of column stress for local failure)</p> <p><b>F<sub>co</sub></b> — column yield stress (upper limit of column stress for primary failure)</p> <p><b>F<sub>cu</sub></b> — ultimate compressive stress</p>	<p><b>F<sub>st</sub></b> — modulus of rupture in torsion</p> <p><b>F<sub>su</sub></b> — ultimate shear stress</p> <p><b>F<sub>t</sub></b> — allowable tensile stress</p> <p><b>f<sub>t</sub></b> — internal (or calculated) tensile stress</p> <p><b>F<sub>tu</sub></b> — ultimate tensile stress</p> <p><b>F<sub>ty</sub></b> — tensile yield stress</p> <p><b>M</b> — bending moment</p> <p><b>P</b> — axial load</p> <p><b>PSI</b> — pounds per square inch</p> <p>or <b>psi</b> — pounds per square inch</p> <p><b>Q</b> — statical moment of a portion of a cross sectional area about the neutral axis of the beam</p> <p><b>G</b> — modulus of elasticity in shear (modulus of rigidity)</p> <p><b>I</b> — moment of inertia of cross sectional area in inch units</p> <p><b>r</b> — radius in inches</p> <p><b>S</b> — shear load</p> <p><b>T</b> — torsional moment, inch units</p> <p><b>t</b> — thickness of wall of tube</p> <p><b>φ</b> — angle of twist of a tube</p> <p><b>Z</b> — section modulus in inch units</p> <p><b>ρ</b> — radius of gyration in inch units</p> <p><b>δ</b> — deformation</p>
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### 2. TABLE OF PROPERTIES OF STEEL AIRCRAFT TUBING

All units in these tables of properties of seamless tubular sections—rounds, streamline, square and elliptical—are inches and pounds except the one column "Wt. per Ft."

The tensile strengths are computed by the formula  $\frac{F_{tu}}{A}$ .

The bending strengths are computed on the basis of the allowable stress obtained from Figure 4-20, page 4-35 of ANC-5, "Strengths of Aircraft Elements." No data are available at this time on the allowable bending stress of 1025 steel.

The torsional strengths are computed on the basis of the allowable shear stress obtained from Figure 4-21 and Figure 4-22 of pages 4-36 and 4-37 of ANC-5, "Strengths of Aircraft Elements."

No data are available at present on the bending and torsional strengths of streamline, square and elliptical tubes. Progress is being made in obtaining these data.

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For explanation of symbols see Sec. III—1

O.D. Inches	Wall		Area	Wt. per ft.	I	Z	$\rho$	Tensile Strength, lbs.			Bending Strength, lbs. in.		Torsional Strength, lbs. in.			
	Decl. Inches	Gage No.						1025 $F_{ru}=55000$ $F_{cy}=36000$	$\times 4130$ $F_{ru}=95000$ $F_{cy}=75000$	$\times 4130$ $F_{ru}=100000$ $F_{cy}=85000$	$\times 4130$ $F_{ru}=95000$ $F_{cy}=75000$	$\times 4130$ $F_{ru}=100000$ $F_{cy}=85000$	1025 $F_{ru}=55000$ $F_{cy}=36000$	$\times 4130$ $F_{ru}=95000$ $F_{cy}=75000$	$\times 4130$ $F_{ru}=100000$ $F_{cy}=85000$	
3/16	.022	24	.0114	.0389	.00004	.0004	.0590	627	1080	1140	51.2	54.2	29	50.1	52.8	
	.028	22	.0140	.0477	.00005	.0005	.0573	770	1330	1400	64.5	68.3	37	65.0	68.4	
	.035	20	.0168	.0570	.00005	.0005	.0553	925	1600	1680	65.0	68.8	39	67.0	70.4	
1/4	.022	24	.0158	.0536	.0001	.0008	.0809	869	1500	1580	99.3	105.5	56	95.8	100.8	
	.028	22	.0195	.0664	.0001	.0009	.0791	1070	1850	1950	115.0	121.5	65	111.0	117.0	
	.035	20	.0236	.0804	.0001	.0011	.0770	1300	2240	2360	142.0	150.0	82	141.0	148.6	
5/16	.022	24	.0201	.0683	.0002	.0013	.1030	1105	1910	2010	154.7	164.0	87	151.0	158.8	
	.028	22	.0250	.0851	.0002	.0016	.1010	1375	2380	2500	200.0	211.0	111	192.6	202.4	
	.035	20	.0305	.1037	.0002	.0019	.0988	1680	2900	3050	243.0	256.5	137	236.4	248.8	
3/8	.022	24	.0244	.0829*	.0003	.0020	.1250	1340	2320	2440	230.0	244.0	130	224.0	236.0	
	.028	22	.0305	.1038	.0004	.0024	.1230	1680	2900	3050	290.5	307.0	163	280.8	296.0	
	.035	20	.0374	.1271	.0005	.0029	.1208	2060	3550	3740	362.5	386.0	203	352.0	370.0	
	.049	18	.0502	.1706	.0006	.0036	.1165	2760	4770	5020	465.0	490.0	287	459.0	483.0	
7/16	.022	24	.0287	.0976	.0006	.0028	.1471	1580	2730	2870	310.5	326.0	176	305.4	321.6	
	.028	22	.0360	.1225	.0007	.0034	.1451	1980	3420	3600	398.0	418.0	228	388.0	408.0	
	.035	20	.0443	.1505	.0009	.0041	.1428	2435	4210	4430	504.5	533.0	281	485.0	510.0	
	.049	18	.0598	.2033	.0011	.0052	.1384	3290	5680	5980	665.5	702.0	375	638.0	671.0	
1/2	.022	24	.0330	.1123	.0010	.0040	.1691	1815	3140	3300	428.0	452.0	246	426.0	448.0	
	.028	22	.0415	.1411	.0012	.0048	.1672	2280	3940	4150	547.0	571.0	309	630.0	562.0	
	.035	20	.0511	.1738	.0014	.0056	.1649	2810	4850	5110	666.0	705.0	376	650.0	683.0	
	.049	18	.0694	.2360	.0018	.0072	.1604	3815	6590	6940	907.0	960.0	508	878.0	924.0	
	.058	17	.0805	.2738	.0020	.0080	.1576	4430	7650	8050	1023.0	1080.0	580	1004.0	1056.0	
5/8	.022	24	.0374	.1270	.0014	.0050	.1913	2055	3550	3740	520.0	550.0	299	516.0	543.0	
	.028	22	.0470	.1596	.0017	.0060	.1892	2585	4460	4700	660.0	696.0	378	652.0	688.0	
	.035	20	.0580	.1972	.0020	.0071	.1869	3190	5510	5800	824.0	866.0	466	806.0	848.0	
	.049	18	.0790	.2687	.0026	.0092	.1824	4350	7500	7900	1140.0	1210.0	638	1102.0	1160.0	
	.058	17	.0919	.3125	.0030	.0107	.1796	5050	8730	9190	1360.0	1433.0	766	1316.0	1384.0	
3/4	.022	24	.0417	.1417	.0019	.0061	.2133	2295	3960	4170	622.0	653.0	356	616.0	647.0	
	.028	22	.0525	.1785	.0023	.0074	.2113	2890	4990	5250	800.0	836.0	456	788.0	830.0	
	.035	20	.0649	.2205	.0028	.0090	.2090	3570	6160	6490	1017.0	1070.0	574	992.0	1044.0	
	.049	18	.0887	.3014	.0037	.0118	.2044	4880	8430	8870	1540.0	1530.0	806	1390.0	1462.0	
	.058	17	.1033	.3512	.0042	.0134	.2015	5690	9820	10330	1675.0	1770.0	938	1620.0	1708.0	
7/8	.022	24	.0460	.1564	.0025	.0073	.2354	2530	4370	4600	730.0	765.0	414	714.0	752.0	
	.028	22	.0580	.1972	.0032	.0093	.2334	3190	5510	5800	977.0	1032.0	562	968.0	1018.0	
	.035	20	.0717	.2439	.0038	.0111	.2310	3940	6810	7170	1232.0	1293.0	702	1214.0	1278.0	
	.049	18	.0983	.3341	.0050	.0145	.2264	5400	9340	9830	1740.0	1832.0	976	1680.0	1770.0	
	.058	17	.1147	.3899	.0057	.0166	.2235	6300	10900	11470	2060.0	2175.0	1148	1984.0	2088.0	
1	.022	24	.0503	.1711	.0033	.0088	.2575	2765	4780	5030	854.0	906.0	484	836.0	880.0	
	.028	22	.0635	.2159	.0041	.0109	.2555	3495	6040	6350	1122.0	1189.0	644	1112.0	1170.0	
	.035	20	.0786	.2673	.0050	.0133	.2531	4320	7470	7860	1450.0	1522.0	828	1426.0	1502.0	
	.049	18	.1079	.3688	.0067	.0179	.2484	5940	10250	10790	2075.0	2210.0	1186	2044.0	2154.0	
	.058	17	.1261	.4287	.0076	.0203	.2455	6940	12000	12610	2480.0	2620.0	1386	2390.0	2518.0	
	.065	16	.1399	.4755	.0083	.0221	.2433	7690	13300	13990	2740.0	2905.0	1532	2644.0	2784.0	
1	.022	24	.0590	.2004	.0054	.0123	.3017	3240	5600	5900	1156.0	1224.0	646	1116.0	1174.0	
	.028	22	.0745	.2533	.0067	.0153	.2996	4100	7080	7450	1530.0	1607.0	866	1498.0	1578.0	
	.035	20	.0924	.3140	.0082	.0187	.2972	5080	8770	9240	1963.0	2065.0	1122	1938.0	2040.0	
	.049	18	.1272	.4323	.0109	.0249	.2925	7000	12100	12720	2840.0	2965.0	1590	2770.0	2880.0	
	.058	17	.1489	.5061	.0125	.0286	.2896	8190	14150	14890	3375.0	3550.0	1900	3276.0	3450.0	
	.065	16	.1654	.5623	.0137	.0313	.2873	9100	15700	16540	3790.0	3990.0	2116	3650.0	3850.0	
	.083	14	.2065	.7021	.0164	.0375	.2814	11350	19600	20650	4730.0	4990.0	2640	4560.0	4800.0	
	1	.028	22	.0855	.2907	.0101	.0202	.3438	4700	8120	8550	1960.0	2050.0	1100	1894.0	1996.0
		.035	20	.1061	.3607	.0124	.0248	.3414	5840	10100	10610	2530.0	2655.0	1440	2486.0	2620.0
.049		18	.1464	.4977	.0166	.0332	.3367	8050	13900	14640	3650.0	3835.0	2084	3596.0	3786.0	
.058		17	.1716	.5835	.0191	.0382	.3337	9440	16300	17160	4350.0	4590.0	2480	4280.0	4510.0	
.065		16	.1909	.6491	.0210	.0420	.3314	10500	18150	19090	4910.0	5200.0	2800	4790.0	5040.0	
.083		14	.2391	.8129	.0253	.0506	.3255	13150	22700	23910	6230.0	6630.0	3440	5940.0	6360.0	
.095		13	.2701	.9182	.0280	.0560	.3217	14850	25650	27010	7050.0	7450.0	3940	6820.0	7160.0	









# Streamline PROPERTIES OF AIRCRAFT STEEL TUBING

Table II

For explanation of symbols see Sec. III—1

Equiv. Round O.D.	Wall		Axes		Area	Wt. per ft.	I Major	Z Major	ρ Major	I Minor	Z Minor	ρ Minor	y	Tensile Strength	
	Dec'l	Gage	Major	Minor										95000	100000
¾	.035	20	1.0114	.4286	.0786	.2673	.0017	.0079	.1486	.0073	.0140	.3046	.520	7460	7860
	.049	18	"	"	.1079	.3668	.0022	.0103	.1435	.0097	.0187	.2998	.519	10230	10790
⅞	.035	20	1.1800	.5000	.0924	.3140	.0028	.0112	.1753	.0118	.0195	.3572	.606	8760	9240
	.049	18	"	"	.1272	.4323	.0037	.0148	.1703	.0158	.0261	.3525	.606	12100	12720
	.058	17	"	"	.1489	.5061	.0042	.0168	.1671	.0182	.0301	.3495	.605	14120	14890
1	.035	20	1.3485	.5714	.1061	.3607	.0043	.0151	.2020	.0178	.0257	.4100	.693	10100	10610
	.049	18	"	"	.1464	.4977	.0057	.0200	.1972	.0239	.0345	.4044	.693	13890	14640
	.058	17	"	"	.1716	.5835	.0065	.0228	.1939	.0278	.0402	.4023	.692	16300	17160
1¼	.035	20	1.5170	.6428	.1199	.4074	.0063	.0196	.2292	.0257	.0329	.4626	.780	11390	11990
	.049	18	"	"	.1656	.5631	.0083	.0258	.2240	.0347	.0445	.4573	.780	15700	16560
	.058	17	"	"	.1944	.6609	.0094	.0292	.2205	.0402	.0516	.4549	.779	18430	19440
	.065	16	"	"	.2165	.7359	.0103	.0320	.2182	.0443	.0569	.4525	.779	20600	21650
1½	.035	20	1.6857	.7143	.1336	.4542	.0088	.0246	.2559	.0355	.0409	.5154	.867	12670	13360
	.049	18	"	"	.1849	.6285	.0116	.0325	.2509	.0482	.0557	.5107	.866	17500	18490
	.058	17	"	"	.2172	.7384	.0133	.0372	.2477	.0560	.0647	.5077	.866	20600	21720
	.065	16	"	"	.2420	.8226	.0145	.0406	.2451	.0618	.0714	.5053	.865	23000	24200
1¾	.035	20	1.8543	.7857	.1473	.5009	.0118	.0300	.2828	.0476	.0549	.5683	.954	14000	14730
	.049	18	"	"	.2041	.6939	.0157	.0400	.2777	.0648	.0680	.5635	.953	19400	20410
	.058	17	"	"	.2400	.8158	.0181	.0461	.2745	.0754	.0791	.5604	.953	22800	24000
	.065	16	"	"	.2675	.9094	.0198	.0504	.2719	.0833	.0875	.5581	.952	25400	26750
1½	.035	20	2.0228	.8571	.1611	.5476	.0154	.0359	.3096	.0621	.0597	.6209	1.040	15300	16110
	.049	18	"	"	.2234	.7593	.0207	.0483	.3045	.0848	.0815	.6162	1.040	21200	22340
	.058	17	"	"	.2627	.8932	.0239	.0555	.3014	.0988	.0951	.6131	1.039	24900	26270
	.065	16	"	"	.2934	.9962	.0262	.0611	.2989	.1093	.1052	.6108	1.039	27800	29340
	.083	14	"	"	.3695	1.256	.0316	.0737	.2923	.1351	.1302	.6047	1.038	35000	36950
1¾	.035	20	2.1913	.9285	.1748	.5943	.0198	.0426	.3365	.0793	.0704	.6736	1.127	16600	17480
	.049	18	"	"	.2426	.8248	.0266	.0573	.3314	.1085	.0964	.6689	1.126	23100	24260
	.058	17	"	"	.2855	.9707	.0308	.0648	.3282	.1266	.1124	.6659	1.126	27100	28550
	.065	16	"	"	.3186	1.083	.0338	.0728	.3256	.1402	.1245	.6634	1.126	30200	31860
	.083	14	"	"	.4021	1.367	.0410	.0883	.3192	.1738	.1545	.6574	1.125	38200	40210
	.095	13	"	"	.4566	1.552	.0453	.0976	.3149	.1949	.1732	.6541	1.125	43400	45660
1¾	.035	20	2.3600	1.0000	.1886	.6411	.0249	.0498	.3633	.0995	.0820	.7263	1.214	17900	18860
	.049	18	"	"	.2618	.8902	.0336	.0672	.3583	.1363	.1123	.7216	1.214	24800	26180
	.058	17	"	"	.3083	1.048	.0389	.0778	.3551	.1592	.1312	.7186	1.213	29300	30830
	.065	16	"	"	.3441	1.170	.0428	.0856	.3525	.1765	.1455	.7163	1.213	32700	34410
	.083	14	"	"	.4347	1.478	.0521	.1042	.3461	.2192	.1809	.7101	1.212	41200	43470
	.095	13	"	"	.4939	1.679	.0577	.1154	.3418	.2463	.2034	.7061	1.211	46800	49390
1¾	.035	20	2.5285	1.0714	.2023	.6878	.0308	.0575	.3902	.1227	.0943	.7789	1.301	19300	20230
	.049	18	"	"	.2811	.9556	.0417	.0779	.3851	.1685	.1296	.7743	1.300	26700	28110
	.058	17	"	"	.3311	1.126	.0482	.0900	.3815	.1969	.1515	.7712	1.300	31400	33110
	.065	16	"	"	.3696	1.257	.0532	.0993	.3794	.2186	.1683	.7690	1.299	35000	36960
	.083	14	"	"	.4673	1.589	.0650	.1214	.3729	.2719	.2093	.7624	1.299	44300	46730
	.095	13	"	"	.5312	1.806	.0722	.1348	.3687	.3598	.2770	.7523	1.299	50600	53120
2	.035	20	2.6970	1.1428	.2161	.7345	.0376	.0658	.4170	.1494	.1076	.8313	1.388	20550	21610
	.049	18	"	"	.3003	1.021	.0510	.0893	.4120	.2053	.1480	.8269	1.387	28600	30030
	.058	17	"	"	.3539	1.203	.0591	.1034	.4088	.2402	.1732	.8238	1.387	32500	35390
	.065	16	"	"	.3951	1.343	.0652	.1141	.4062	.2668	.1925	.8217	1.386	37500	39510
	.083	14	"	"	.4999	1.699	.0799	.1398	.3998	.3325	.2399	.8156	1.386	47400	49990
	.095	13	"	"	.5685	1.933	.0889	.1556	.3954	.3744	.2703	.8115	1.385	54000	56850
2¼	.049	18	3.0343	1.2857	.3388	1.152	.0735	.1144	.4657	.2946	.1887	.9325	1.561	32200	33880
	.058	17	"	"	.3994	1.358	.0854	.1329	.4625	.3450	.2212	.9294	1.560	37900	39940
	.065	16	"	"	.4462	1.517	.0944	.1469	.4600	.3835	.2458	.9271	1.560	42500	44620
	.083	14	"	"	.5650	1.921	.1161	.1807	.4533	.4794	.3075	.9223	1.559	53600	56500
	.095	13	"	"	.6432	2.186	.1298	.2020	.4492	.5409	.3470	.9170	1.559	61000	64320
	.120	11	"	"	.8030	2.730	.1556	.2421	.4402	.6629	.4255	.9086	1.558	76200	80300



# Streamline PROPERTIES OF AIRCRAFT STEEL TUBING

Table II

For explanation of symbols see Sec. III—1

Equiv. Round O.D.	Wall		Axes		Area	Wt. per ft.	I Major	Z Major	ρ Major	I Minor	Z Minor	ρ Minor	y	Tensile Strength	
	Dec'l	Gage	Major	Minor										95000	100000
2½	.049	18	3.3713	1.4285	.3773	1.283	.1018	.1425	.5194	.4063	.2343	1.0377	1.734	35800	37730
	.058	17	"	"	.4450	1.513	.1186	.1660	.5163	.4764	.2747	1.0346	1.734	42300	44500
	.065	16	"	"	.4972	1.690	.1312	.1837	.5137	.5300	.3057	1.0325	1.734	47300	49720
	.083	14	"	"	.6302	2.143	.1621	.2269	.5071	.6640	.3832	1.0265	1.733	59900	63020
	.095	13	"	"	.7178	2.440	.1815	.2541	.5028	.7504	.4333	1.0224	1.732	68100	71780
	.120	11	"	"	.8972	3.050	.2188	.3063	.4938	.9226	.5329	1.0141	1.731	85100	89720
2¾	.049	18	3.7085	1.5714	.4158	1.414	.1366	.1739	.5732	.5434	.2848	1.1432	1.908	39500	41580
	.058	17	"	"	.4905	1.668	.1593	.2028	.5699	.6372	.3340	1.1398	1.908	46600	49050
	.065	16	"	"	.5483	1.864	.1765	.2247	.5674	.7099	.3723	1.1379	1.907	62100	54830
	.083	14	"	"	.6954	2.364	.2188	.2785	.5609	.8907	.4673	1.1318	1.906	66100	69540
	.095	13	"	"	.7924	2.694	.2455	.3125	.5566	1.0081	.5289	1.1279	1.906	75300	79240
	.120	11	"	"	.9915	3.371	.2973	.3785	.5476	1.2427	.6523	1.1196	1.905	94900	99150
3	.058	17	4.0455	1.7142	.5361	1.822	.2085	.2433	.6236	.8318	.3997	1.2456	2.081	51000	53610
	.065	16	"	"	.5993	2.037	.2312	.2698	.6211	.9265	.4452	1.2434	2.081	56800	59930
	.083	14	"	"	.7606	2.586	.2872	.3352	.6145	1.1645	.5599	1.2373	2.080	72400	76060
	.095	13	"	"	.8670	2.947	.3229	.3768	.6103	1.3184	.6338	1.2332	2.080	82300	86700
	.120	11	"	"	1.0857	3.691	.3925	.4580	.6013	1.6289	.7839	1.2249	2.078	103000	108570
	.156	¾	"	"	1.3959	4.746	.4824	.5630	.5879	2.0497	.9869	1.2118	2.077	132500	139590
3¼	.058	17	4.3828	1.8571	.5816	1.977	.2668	.2873	.6773	1.0617	.4708	1.3511	2.255	55350	58160
	.065	16	"	"	.6504	2.211	.2961	.3189	.6747	1.1830	.5248	1.3487	2.254	61900	65040
	.083	14	"	"	.8258	2.807	.3689	.3973	.6684	1.4886	.6604	1.3426	2.254	78500	82580
	.095	13	"	"	.9416	3.201	.4150	.4470	.6639	1.6873	.7489	1.3386	2.253	89500	94160
	.120	11	"	"	1.1800	4.011	.5062	.5452	.6549	2.0887	.9275	1.3304	2.252	112000	118000
	.156	¾	"	"	1.5186	5.163	.6251	.6732	.6416	2.6353	1.1707	1.3173	2.251	144000	151860
3½	.065	16	4.7200	2.0000	.7014	2.385	.3723	.3723	.7286	1.4831	.6108	1.4541	2.428	66700	70140
	.083	14	"	"	.8910	3.029	.4645	.4645	.7220	1.8686	.7699	1.4481	2.427	84600	89100
	.095	13	"	"	1.0162	3.455	.5234	.5234	.7177	2.1194	.8733	1.4442	2.427	96550	101620
	.120	11	"	"	1.2742	4.332	.6400	.6400	.7087	2.6269	1.0828	1.4358	2.426	121300	127420
	.156	¾	"	"	1.6414	5.580	.7934	.7934	.6952	3.3221	1.3705	1.4227	2.424	156200	164140
	3¾	.065	16	5.0570	2.1428	.7525	2.558	.4604	.4297	.7822	1.8299	.7033	1.5594	2.602	71500
.083		14	"	"	.9562	3.251	.5754	.5371	.7757	2.3073	.8871	1.5534	2.601	91000	95620
.095		13	"	"	1.0908	3.708	.6490	.5777	.7713	2.6195	1.0075	1.5497	2.600	103900	109080
.120		11	"	"	1.3685	4.652	.7955	.7425	.7624	3.2505	1.2502	1.5412	2.600	130000	136850
.156		¾	"	"	1.7641	5.997	.9894	.9235	.7489	4.1192	1.5855	1.5281	2.598	167800	176410
4		.065	16	5.3940	2.2856	.8035	2.732	.5614	.4912	.8359	2.2270	.8025	1.6648	2.775	76300
	.083	14	"	"	1.0214	3.472	.7024	.6146	.8293	2.8161	1.0148	1.6604	2.775	97200	102140
	.095	13	"	"	1.1655	3.962	.7933	.6941	.8250	3.1917	1.1506	1.6549	2.774	110800	116550
	.120	11	"	"	1.4627	4.973	.9739	.8522	.8160	3.9661	1.4303	1.6467	2.773	139000	146270
	.156	¾	"	"	1.8867	6.414	1.2153	1.0634	.8026	5.0332	1.8164	1.6333	2.771	179000	188670
	.188	¾	"	"	2.2457	7.635	1.4109	1.2345	.7926	5.9364	2.1431	1.6259	2.770	213500	224570
4¼	.065	16	5.7313	2.4285	.8546	2.905	.6761	.5568	.8895	2.6784	.9082	1.7704	2.949	81100	85460
	.083	14	"	"	1.0866	3.694	.8473	.6978	.8830	3.3817	1.1471	1.7641	2.948	103000	108660
	.095	13	"	"	1.2400	4.216	.9574	.7885	.8787	3.8426	1.3035	1.7604	2.948	118000	124000
	.120	11	"	"	1.5570	5.293	1.1779	.9701	.8698	4.7789	1.6216	1.7519	2.947	148000	155700
	.156	¾	"	"	2.0095	6.832	1.4729	1.2131	.8573	6.0748	2.0628	1.7387	2.945	191000	200950
	.188	¾	"	"	2.3930	8.135	1.7144	1.4120	.8464	7.1732	2.4365	1.7314	2.944	227500	239300
4½	.083	14	6.0683	2.5713	1.1517	3.915	1.0105	.7860	.9367	4.0256	1.2894	1.8696	3.122	109500	115170
	.095	13	"	"	1.3147	4.469	1.1429	.8889	.9324	4.5759	1.4662	1.8656	3.121	125000	131470
	.120	11	"	"	1.6512	5.613	1.4082	1.0953	.9235	5.6952	1.8254	1.8572	3.120	157000	165120
	.156	¾	"	"	2.1322	7.249	1.7650	1.3728	.9098	7.2508	2.3247	1.8441	3.119	202500	213220
	.188	¾	"	"	2.5403	8.636	2.0580	1.6007	.9001	8.5740	2.7498	1.8372	3.118	241500	254030
	.219	¾	"	"	2.9421	10.01	2.3221	1.8061	.8884	9.8001	3.1451	1.8251	3.116	280000	294210
4¾	.083	14	6.4055	2.7142	1.2169	4.137	1.1938	.8797	.9910	4.7471	1.4403	1.9751	3.296	115600	121690
	.095	13	"	"	1.3893	4.723	1.3509	.9955	.9861	5.3978	1.6382	1.9711	3.295	132000	138930
	.120	11	"	"	1.7455	5.934	1.6668	1.2283	.9772	6.7256	2.0418	1.9629	3.294	166000	174550
	.156	¾	"	"	2.2550	7.666	2.0932	1.5425	.9634	8.5673	2.6017	1.9492	3.293	214000	225500
	.188	¾	"	"	2.6875	9.137	2.4455	1.8021	.9539	10.1398	3.0811	1.9424	3.291	255500	268750
	.219	¾	"	"	3.1139	10.59	2.7633	2.0363	.9420	11.6100	3.5289	1.9309	3.290	296000	311390



For explanation of symbols see Sec. III-I

Equiv. Round O.D.	Wall		Axes		Area	Wt. per ft.	I Major	Z Major	$\rho$ Major	I Minor	Z Minor	$\rho$ Minor	Tensile Strength	
	Dec'l	Gage	Major	Minor									95000	100000
3/4	.035	20	.9728	.4864	.0786	.2673	.0023	.0094	.1709	.0069	.0143	.2971	7460	7860
7/8	.035	20	1.1348	.5674	.0924	.3140	.0037	.0132	.2013	.0112	.0198	.3485	8760	9240
	.049	18	"	"	.1272	.4323	.0049	.0173	.1965	.0150	.0265	.3437	12100	12720
1	.035	20	1.2972	.6486	.1061	.3607	.0057	.0176	.2314	.0170	.0262	.4003	10100	10610
	.049	18	"	"	.1464	.4977	.0075	.0233	.2271	.0229	.0353	.3954	13900	14640
1 1/8	.035	20	1.4592	.7296	.1199	.4074	.0083	.0226	.2623	.0245	.0335	.4518	11380	11990
	.049	18	"	"	.1656	.5631	.0110	.0301	.2576	.0331	.0453	.4469	15700	16560
1 1/4	.035	20	1.6212	.8106	.1336	.4542	.0115	.0287	.2930	.0339	.0418	.5034	12660	13360
	.049	18	"	"	.1849	.6285	.0153	.0378	.2880	.0459	.0567	.4986	17510	18490
1 3/8	.058	17	"	"	.2172	.7384	.0176	.0435	.2849	.0533	.0657	.4952	20300	21720
	.035	20	1.7824	.8912	.1473	.5009	.0154	.0345	.3231	.0453	.0508	.5546	14000	14730
1 1/2	.049	18	"	"	.2041	.6939	.0207	.0464	.3182	.0616	.0692	.5496	19400	20410
	.058	17	"	"	.2400	.8158	.0238	.0535	.3151	.0716	.0804	.5463	22800	24000
1 5/8	.035	20	1.9456	.9728	.1611	.5476	.0202	.0415	.3539	.0587	.0604	.6038	15300	16110
	.049	18	"	"	.2234	.7593	.0272	.0559	.3489	.0809	.0831	.6016	21200	22340
1 7/8	.058	17	"	"	.2627	.8932	.0314	.0646	.3459	.0941	.0967	.5983	25000	26270
	.065	16	"	"	.2930	.9962	.0346	.0711	.3435	.1041	.1070	.5960	27800	29300
2	.049	18	2.1076	1.0538	.2426	.8248	.0349	.0663	.3795	.1035	.0982	.6531	23100	24260
	.058	17	"	"	.2855	.9707	.0404	.0767	.3763	.1206	.1145	.6492	27000	28550
2 1/4	.065	16	"	"	.3186	1.083	.0445	.0845	.3739	.1336	.1267	.6467	30300	31860
	.049	18	2.2700	1.1350	.2618	.8902	.0440	.0776	.4101	.1301	.1146	.7050	24800	26180
2 1/2	.058	17	"	"	.3083	1.048	.0510	.0900	.4069	.1518	.1338	.7017	29300	30830
	.065	16	"	"	.3441	1.170	.0563	.0992	.4045	.1682	.1482	.6992	32800	34410
2 3/4	.049	18	2.4320	1.2160	.2811	.9556	.0546	.0897	.4405	.1608	.1323	.7564	26750	28110
	.058	17	"	"	.3311	1.126	.0633	.1042	.4374	.1878	.1545	.7532	31400	33110
3	.065	16	"	"	.3696	1.257	.0699	.1150	.4347	.2083	.1713	.7507	35100	36960
	.083	14	"	"	.4673	1.589	.0859	.1413	.4287	.2589	.2129	.7443	44400	46730
3 1/4	.049	18	2.5940	1.2970	.3003	1.021	.0666	.1027	.4710	.1961	.1512	.8080	28600	30030
	.058	17	"	"	.3539	1.203	.0775	.1194	.4681	.2292	.1767	.8047	33600	35390
3 1/2	.065	16	"	"	.3951	1.343	.0856	.1320	.4658	.2543	.1961	.8023	37500	39510
	.083	14	"	"	.4999	1.699	.1054	.1625	.4591	.3166	.2441	.7958	47400	49990
3 3/4	.095	13	"	"	.5685	1.933	.1177	.1815	.4551	.3563	.2747	.7917	53900	56850
	.058	17	2.9184	1.4592	.3994	1.358	.1117	.1531	.5289	.3293	.2257	.9080	37800	39940
4	.065	16	"	"	.4462	1.517	.1237	.1695	.5266	.3658	.2507	.9055	42300	44620
	.083	14	"	"	.5650	1.921	.1529	.2095	.5202	.4568	.3130	.8991	53600	56500
4 1/4	.095	13	"	"	.6432	2.186	.1713	.2348	.5160	.5150	.3529	.8948	61100	64320
	.058	17	3.2428	1.6214	.4450	1.513	.1549	.1911	.5900	.4550	.2806	1.0112	42300	44500
4 1/2	.065	16	"	"	.4972	1.690	.1716	.2117	.5875	.5060	.3121	1.0088	47200	49720
	.083	14	"	"	.6302	2.143	.2129	.2626	.5812	.6332	.3905	1.0024	59900	63020
4 3/4	.095	13	"	"	.7178	2.440	.2390	.2948	.5771	.7150	.4410	.9981	68200	71780
	.120	11	"	"	.8972	3.050	.2900	.3577	.5685	.8781	.5416	.9893	85300	89720
5	.065	16	3.5668	1.7834	.5483	1.864	.2306	.2586	.6488	.6778	.3800	1.1118	52100	54830
	.083	14	"	"	.6954	2.364	.2868	.3216	.6422	.8498	.4765	1.1054	66100	69540
5 1/4	.095	13	"	"	.7924	2.694	.3225	.3617	.6380	.9607	.5387	1.1001	75100	79240
	.120	11	"	"	.9915	3.371	.3927	.4404	.6294	1.1829	.6633	1.0923	94200	99150
5 1/2	.156	9	"	"	1.2732	4.328	.4841	.5430	.6167	1.4820	.8310	1.0789	121000	127320
	.065	16	3.8912	1.9456	.5993	2.037	.3019	.3103	.7097	.8848	.4548	1.2151	56800	59930
5 3/4	.083	14	"	"	.7606	2.586	.3763	.3868	.7034	1.1111	.5711	1.2086	72200	76060
	.095	13	"	"	.8670	2.947	.4238	.4357	.6992	1.2576	.6464	1.2044	82500	86700
6	.120	11	"	"	1.0857	3.691	.5176	.5321	.6905	1.5517	.7976	1.1955	103000	108570
	.156	9	"	"	1.3959	4.746	.6409	.6589	.6776	1.9501	1.0024	1.1820	132200	139590
6 1/4	.188	7	"	"	1.6567	5.632	.7396	.7603	.6682	2.2803	1.1721	1.1732	157000	165670
	.065	16	4.2152	2.1076	.6504	2.211	.3862	.3664	.7705	1.1301	.5362	1.3180	61900	65040
6 1/2	.083	14	"	"	.8258	2.807	.4823	.4576	.7642	1.4208	.6742	1.3117	78400	82580
	.095	13	"	"	.9416	3.201	.5438	.5161	.7600	1.6095	.7637	1.3074	89400	94160
6 3/4	.120	11	"	"	1.1800	4.011	.6660	.6320	.7513	1.9896	.9441	1.2985	112000	118000
	.156	9	"	"	1.5186	5.163	.8279	.7856	.7383	2.5071	1.1896	1.2849	143900	151860
7	.188	7	"	"	1.8040	6.133	.9586	.9096	.7221	2.9385	1.3943	1.2763	171000	180400
	.065	16	4.5396	2.2698	.7014	2.385	.4851	.4274	.8316	1.4172	.6244	1.4215	66600	70140
7 1/4	.083	14	"	"	.8910	3.029	.6068	.5347	.8253	1.7839	.7859	1.4150	84600	89100
	.095	13	"	"	1.0162	3.455	.6850	.6036	.8217	2.0223	.8909	1.4107	96000	101620
7 1/2	.120	11	"	"	1.2742	4.332	.8408	.7408	.8123	2.5037	1.1031	1.4017	121000	127420
	.156	9	"	"	1.6414	5.580	1.0486	.9239	.7993	3.1620	1.3931	1.3880	156000	164140
7 3/4	.188	7	"	"	1.9512	6.663	1.2177	1.0729	.7900	3.7136	1.6361	1.3796	185200	195120



For explanation of symbols see Sec. III—1

Size	Wall		Area	Wt. per ft.	I	Z	$\rho$	Tensile Strength	
	Dec'l	Gage						95000	100000
3/8	.022	24	.0294	.0998	.0006	.0031	.1415	2795	2940
	.028	22	.0368	.1252	.0007	.0038	.1396	3500	3680
	.035	20	.0453	.1589	.0009	.0045	.1372	43100	4530
	.049	18	.0612	.2081	.0011	.0057	.1318	5820	6120
1/2	.022	24	.0396	.1348	.0016	.0065	.2023	3760	3960
	.028	22	.0499	.1697	.0020	.0079	.1991	4740	4990
	.035	20	.0616	.2096	.0024	.0094	.1956	5850	6160
	.049	18	.0841	.2861	.0030	.0120	.1892	8000	8410
	.058	17	.0980	.3330	.0033	.0134	.1853	9300	9800
	.065	16	.1084	.3684	.0035	.0141	.1804	10300	10840
5/8	.022	24	.0483	.1641	.0033	.0104	.2600	4640	4830
	.028	22	.0630	.2143	.0040	.0129	.2526	5990	6300
	.035	20	.0780	.2652	.0048	.0155	.2489	7400	7800
	.049	18	.1071	.3640	.0063	.0201	.2420	10200	10710
	.058	17	.1251	.4253	.0071	.0227	.2379	11900	12510
	.065	16	.1388	.4717	.0077	.0245	.2348	13200	13880
3/4	.028	22	.0761	.2588	.0064	.0168	.2890	7240	7610
	.035	20	.0944	.3209	.0078	.0206	.2866	8950	9440
	.049	18	.1300	.4419	.0103	.0274	.2816	12350	13000
	.058	17	.1522	.5175	.0118	.0313	.2784	14500	15220
	.065	16	.1692	.5751	.0129	.0342	.2760	16100	16920
	.083	14	.2113	.7184	.0154	.0408	.2700	20100	21130
7/8	.028	22	.0892	.3033	.0103	.0236	.3390	8470	8920
	.035	20	.1108	.3766	.0125	.0288	.3364	10510	11080
	.049	18	.1528	.5193	.0168	.0386	.3320	14500	15280
	.058	17	.1794	.6098	.0193	.0445	.3283	17100	17940
	.065	16	.1996	.6785	.0212	.0487	.3528	19000	19960
	.083	14	.2502	.8504	.0255	.0588	.3194	23800	25020
1	.028	22	.1023	.3479	.0154	.0309	.3883	9720	10230
	.035	20	.1271	.4322	.0190	.0380	.3867	12100	12710
	.049	18	.1758	.5978	.0256	.0511	.3813	16700	17580
	.058	17	.2065	.7020	.0295	.0591	.3782	19600	20650
	.065	16	.2300	.7819	.0324	.0649	.3756	21900	23000
	.083	14	.2890	.9824	.0394	.0788	.3693	27500	28900
.095	13	.3272	1.112	.0436	.0873	.3652	31100	32720	
1 1/8	.035	20	.1435	.4878	.0275	.0489	.4340	13650	14350
	.049	18	.1988	.6757	.0369	.0657	.4311	18900	19880
	.058	17	.2336	.7942	.0427	.0759	.4265	22200	23360
	.065	16	.2604	.8852	.0471	.0838	.4241	24800	26040
	.083	14	.3278	1.114	.0576	.1024	.4191	31100	32780
	.095	13	.3716	1.263	.0640	.1138	.4151	35700	37160
.120	11	.4600	1.564	.0760	.1351	.4064	43700	46000	
1 1/4	.035	20	.1599	.5435	.0378	.0604	.4891	15200	15990
	.049	18	.2217	.7536	.0513	.0821	.4831	21100	22170
	.058	17	.2608	.8865	.0595	.0952	.4778	24800	26080
	.065	16	.2908	.9886	.0657	.1051	.4753	27600	29080
	.083	14	.3666	1.246	.0806	.1290	.4960	34800	36660
	.095	13	.4161	1.414	.0899	.1438	.4647	39600	41610
.120	11	.5161	1.755	.1074	.1718	.4561	49000	51610	
1 3/8	.035	20	.1763	.5992	.0506	.0736	.5360	16800	17630
	.049	18	.2446	.8316	.0689	.1003	.5309	23200	24460
	.058	17	.2879	.9787	.0801	.1166	.5276	27400	28790
	.065	16	.3212	1.092	.0886	.1288	.5251	30500	32120
	.083	14	.4055	1.378	.1091	.1587	.5187	38600	40550
	.095	13	.4605	1.566	.1219	.1773	.5145	43800	46050
.120	11	.5723	1.946	.1464	.2130	.5058	54400	57230	

For explanation of symbols see Sec. III—1

Side	Wall		Area	Wt. per ft.	I	Z	$\rho$	Tensile Strength	
	Dec'l	Gage						95000	100000
1½	.035	20	.1926	.6549	.0661	.0881	.5859	18300	19260
	.049	18	.2675	.9095	.0902	.1203	.5807	25400	26750
	.058	17	.3150	1.071	.1051	.1401	.5776	29900	31500
	.065	16	.3516	1.195	.1162	.1550	.5750	33400	35160
	.083	14	.4443	1.510	.1436	.1915	.5686	42200	44430
	.095	13	.5050	1.717	.1608	.2144	.5643	48000	50500
	.120	11	.6284	2.136	.1940	.2586	.5556	59600	62840
1¾	.049	18	.3134	1.065	.1451	.1658	.6804	29800	31340
	.058	17	.3693	1.255	.1691	.1932	.6766	35100	36930
	.065	16	.4124	1.402	.1877	.2146	.6747	39200	41240
	.083	14	.5220	1.774	.2331	.2663	.6682	49600	52200
	.095	13	.5938	2.019	.2618	.2992	.6640	56400	59380
	.120	11	.7307	2.484	.3175	.3628	.6592	69500	73070
2	.049	18	.3592	1.221	.2180	.2180	.7790	34100	35920
	.058	17	.4236	1.440	.2557	.2557	.7769	40250	42360
	.065	16	.4732	1.609	.2838	.2838	.7744	45000	47320
	.083	14	.5996	2.038	.3536	.3536	.7679	56900	59960
	.095	13	.6827	2.321	.3981	.3981	.7636	64900	68270
	.120	11	.8530	2.900	.4859	.4856	.7548	81000	85300
	.156	¾	1.0912	3.710	.6010	.6010	.7422	104000	109120
2¼	.049	18	.4051	1.377	.3080	.2738	.8721	38550	40510
	.058	17	.4778	1.624	.3607	.3207	.8689	45400	47780
	.065	16	.5341	1.816	.4000	.3556	.8654	50700	53410
	.083	14	.6773	2.302	.5007	.4451	.8598	64400	67730
	.095	13	.7716	2.623	.5640	.5013	.8549	73400	77160
	.120	11	.9652	3.281	.6918	.6149	.8466	91600	96520
	.156	¾	1.2372	4.206	.8765	.7791	.8418	117500	123720
2½	.058	17	.5321	1.809	.5074	.4035	.9757	50600	53210
	.065	16	.5949	2.022	.5643	.4488	.9739	56400	59490
	.083	14	.7549	2.566	.7071	.5625	.9678	71600	75490
	.095	13	.8605	2.925	.7981	.6530	.9631	81800	86050
	.120	11	1.0775	3.663	.9809	.7806	.9541	102500	107750
	.156	¾	1.3831	4.702	1.2256	.9760	.9413	131500	138310
	.188	⅝	1.6479	5.602	1.4257	1.3183	.9301	156500	164790
2¾	.058	17	.5864	1.993	.6792	.4940	1.0763	55700	58640
	.065	16	.6557	2.229	.7559	.5497	1.0737	62400	65570
	.083	14	.8326	2.830	.9481	.6896	1.0671	79000	83260
	.095	13	.9494	3.227	1.0724	.7799	1.0628	90100	94940
	.120	11	1.1898	4.045	1.3213	.8653	1.0541	113000	118980
	.156	¾	1.5291	5.198	1.6569	1.1121	1.0410	145000	152910
	.188	⅝	1.8238	6.200	1.9334	1.3264	1.0296	173500	182380
3	.065	16	.7166	2.436	.9860	.6573	1.1730	68100	71660
	.083	14	.9102	3.094	1.2394	.8263	1.1669	87500	91020
	.095	13	1.0383	3.530	1.4032	.9355	1.1625	98500	103830
	.120	11	1.3021	4.426	1.7325	1.1550	1.1544	124000	130210
	.156	¾	1.6750	5.694	2.1793	1.4529	1.1406	159200	167500
	.188	⅝	1.9997	6.798	2.5503	1.7002	1.1381	190000	199970
	.219	¾	2.3081	7.847	2.8873	1.9249	1.1184	219000	230810



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### TABLE V

MECHANICAL PROPERTIES OF MATERIALS			1025 Steel		
CONDITION			Before Welding	After Welding (Properties in Affected Zone)	
Specification			Army*	Tube 57-108-1*	Tube 57-180-1*
			Navy*	Tube 49-T-1*	Tube 49-T-1*
TENSION	$F_{tu}$	Ultimate Stress,	psi	55,000	45,000
	$F_{ty}$	Yield Stress,	psi	36,000	
	$F_{tp}$	Proportional Limit,	psi	25,000	
	E	Modulus of Elasticity,	psi	28,000,000	
COMPRESSION	$F_{cu}$	Ultimate (block) Stress,	psi	55,000	
	$F_{cy}$	Yield Stress,	psi	36,000	
	$F_{cp}$	Proportional Limit,	psi	25,000	
	$F_{co}$	Column Yield Stress,	psi	36,000	
	$E_c$	Modulus of Elasticity,	psi	28,000,000	
SHEAR	$F_{su}$	Ultimate Stress,	psi	35,000	
	$F_{st}$	Torsional Modulus of Rupture,	psi	50,000	
	$F_{sp}$	Proportional Limit (torsion),	psi	20,000	
	G	Modulus of Rigidity (torsion),	psi	10,000,000	
BEARING	$F_{br}$	Ultimate Stress,	psi	90,000	
FATIGUE	$F_{be}$	Bending Endurance Limit, (300,000,000 cycles of completely reversed stress)	psi	25,000	
	w	Specific weight,      0.2833 lb./cu. in.      490 lb./cu. ft.			
		Nominal Chemical Composition 0.25% C, .65 Mn, .045% P (max.), .050% S (max)			

**REMARKS:**

1. Where joints with tapered welds at angles of 30 degrees or less to the center line, or fish-mouth welds formed by cuts of 60 degrees or less are used, the allowable tensile stress near the welding can be assumed to be 50,000 psi.
2. In welded structures where seven or more members converge, the allowable stress shall be determined by dividing the normal allowable stress by a materials factor of 1.5, unless the joint is reinforced in a manner for which specific authority has been obtained from the licensing or procuring agency. A tube that is continuous through a joint should be assumed as two members.

\*Note—The new Aeronautical Board Specification for 1025 Carbon Steel tubing is AN-WW-T-846. It is now in use but there is no change in physical properties affecting the values in this table.

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TABLE VI

MECHANICAL PROPERTIES AND MATERIALS			Alloy Steels -X4130			
Condition				Normalized Plate Tube and Bar—.188" Thick and Under (X-4130)	Near Welding <sup>2</sup> when Welded after Heat Treatment (X-4130) Special	Normalized <sup>1</sup> Tubes—.188" Thick and Under (X-4130) Special
Specification			Army*	57-180-2D for Tubing		
			Navy*	Rd. Tubing 44T18 Str. Tubing 44T17		
TENSION	F <sub>tu</sub>	Ultimate Stress,	psi	95,000	84,000	100,000
	F <sub>ty</sub>	Yield Stress,	psi	75,000		85,000
	F <sub>tp</sub>	Proportional Limit,	psi			
	E	Modulus of Elasticity,	psi	29,000,000	29,000,000	29,000,000
		Elongation in 2 in., %				12
COMPRESSION	F <sub>cu</sub>	Ultimate (block) Stress,	psi	95,000	76,000	100,000
	F <sub>cy</sub>	Yield Stress,	psi	75,000		85,000
	F <sub>cp</sub>	Proportional Limit,	psi			
	F <sub>co</sub>	Column Yield Stress,	psi	79,500		90,100
	E <sub>c</sub>	Modulus of Elasticity,	psi	29,000,000	29,000,000	29,000,000
SHEAR	F <sub>su</sub>	Ultimate Stress,	psi	55,000	52,500	58,000
	F <sub>st</sub>	Torsional Modulus of Rupture	psi	80,000	73,500	84,000
	F <sub>sp</sub>	Proportional Limit (torsion)	psi	40,000		
	G	Modulus of Rigidity (torsion)	psi	11,000,000	11,000,000	11,000,000
BEARING	F <sub>br</sub>	Ultimate Stress,	psi	140,000	130,000	147,000
FATIGUE	F <sub>be</sub>	Bending Endurance Limit, (300,000,000 cycles of completely reversed stress)	psi	45,000		
w.	Specific Weight,		0.2833	lb./cu. in.	490	lb./cu. ft.

REMARKS:

1. The properties in this line are for use in connection with *civil* aircraft only. Their use is permissible provided that the tensile properties stated are guaranteed by the manufacturer of the tubing. These properties apply only to tubing "as received."
2. In welded structures where seven or more members converge, the allowable stress shall be determined by dividing the normal allowable stress by a materials factor of 1.5, unless the joint is reinforced in a manner for which specific authority has been obtained from the licensing or procuring agency. A tube that is continuous through a joint should be assumed as two members.

\*Note—The new Aeronautical Board Specification for X4130 tubing will be issued in March 1941 as AN-WW-T-850, and in all probability will be used for aircraft requirements by all government agencies. It does not in any way affect the physical values in this table.



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### 3. COMMONLY USED FORMULAS

It is assumed that engineers using this handbook are thoroughly familiar with the basic principles of the strength of materials such as found in any textbook on the subject. The following formulas of strength of materials are listed for reference purposes. The sign conventions generally accepted in their use are that quantities associated with tensile action (load, stress, strain, etc.) are considered as negative. When compressive action is of primary interest, however, it is sometimes convenient to consider the associated quantities to be positive.

#### Simple Unit Stresses

$$3:1 \quad f_t = \frac{P}{A} \text{ (tension)}$$

$$3:2 \quad f_c = \frac{P}{A} \text{ (compression)}$$

$$3:3 \quad f_b = \frac{My}{I} = \frac{M}{Z}$$

$$3:4 \quad f_s = \frac{S}{A} \text{ (average direct shear stress)}$$

$$3:5 \quad f_s = \frac{SQ}{Ib} \text{ (longitudinal or transverse shear stress)}$$

$$3:6 \quad f_s = \frac{Tr}{I_p} \text{ (shear stress in round tubes due to torsion)}$$

$$3:7 \quad f_s = \frac{T}{2At} \text{ (shear stress due to torsion in thin-walled structures of closed section. Note that } A \text{ is the area enclosed by the median line of the section)}$$

#### Combined Stresses

$$3:8 \quad f_n = f_c + f_b \text{ (compression and bending)}$$

$$3:9 \quad f_{s \max} = \sqrt{f_s^2 + \left(\frac{f_n}{2}\right)^2} \text{ (compression, bending, and torsion)}$$

$$3:10 \quad f_{n \max} = \frac{f_n}{2} + f_{s \max}$$

#### Deflections (axial)

$$3:11 \quad e = \frac{\delta}{L} \text{ (unit deformation or strain)}$$

$$3:12 \quad E = \frac{f}{e} \text{ (This equation applies when } E \text{ is to be found from tests in which } f \text{ and } e \text{ are measured)}$$

$$3:13 \quad \delta = eL = \frac{f}{E} L = \frac{PL}{AE} \text{ (This equation applies when the deflection is to be calculated using a known value of } E)$$

$$3:14 \quad \phi = \frac{TL}{GJ} \text{ (Used when torque } T \text{ is constant over length } L)$$

#### Basic Column Formulas

$$3:15 \quad F_c = \frac{c\pi^2 E}{(L/\rho)^2} \text{ (Euler formula for long columns)}$$

$$= \frac{\pi^2 E}{(L'/\rho)^2} \text{ where } L' = \frac{L}{c}$$

$$3:16 \quad F_c = F_{co} \left[ 1 - K \left( \frac{L'/\rho}{\pi\sqrt{E/F_{co}}} \right)^n \right] \text{ (General parabolic formula)}$$

$$3:17 \quad F_c = F_{co} \left[ 1 - \frac{F_{co} (L'/\rho)^2}{4\pi^2 E} \right] \text{ (2.0 parabola—Johnson formula)}$$

$$3:18 \quad F_c = F_{co} \left[ 1 - .3027 \left( \frac{L'/\rho}{\pi\sqrt{E/F_{co}}} \right)^{1.5} \right] \text{ (1.5 parabola)}$$

$$3:19 \quad F_c = F_{co} \left[ 1 - .385 \left( \frac{L'/\rho}{\pi\sqrt{E/F_{co}}} \right) \right] \text{ (1.0 parabola—straight line formula)}$$

#### Basic Column Formulas (non-dimensional)

$$3:20 \quad R_a = \frac{F_c}{F_{co}} \text{ (Allowable stress ratio)}$$

$$3:21 \quad B = \frac{L'/\rho}{\pi\sqrt{E/F_{co}}} \text{ (slenderness ratio factor)}$$

$$3:22 \quad R_a = \left( \frac{1}{B} \right)^2 \text{ (Euler formula)}$$

$$3:23 \quad R_a = 1 - KB^n \text{ (general parabolic formula)}$$

$$3:24 \quad R_a = 1 - .25B^2 \text{ (2.0 parabola—Johnson formula)}$$

$$3:25 \quad R_a = 1 - .3027B^{1.5} \text{ (1.5 parabola)}$$

$$3:26 \quad R_a = 1 - .285B \text{ (1.0 parabola—straight line formula)}$$

### 4. COLUMNS AND COLUMN CURVES

A theoretical treatment of columns can be found in standard textbooks on the strength of materials. The problems confronting the designer include, however, many points which are not well defined by theory and which frequently cause some confusion. These will be considered in this section.

#### Primary Instability Failure

**General:** A column may fail through primary instability by bending laterally or by twisting about some axis parallel to its own axis. This latter type of primary failure is particularly common to columns having unsymmetrical open sections. The twisting failure of a closed section column such as a tube is precluded by its inherently high torsional rigidity.

**Long Columns:** The Euler formula for long columns which fail by lateral bending is given by Eq. 3:15. No explanation of this classical formula need be offered, as its derivation can be found in many standard textbooks on the strength of materials. The value to be used for the restraint coefficient,  $c$ , depends on the degree of end fixation. Definite rules as to the maximum value which may be assumed are given in the specific airworthiness requirements of the Government services. The true significance of the restraint coefficient is best understood by considering the end restraint as modifying the effective column length, as indicated in Eq. 3:15. For a pin-ended column having zero end restraint  $c = 1.0$  and  $L' = L$ . A fixity coefficient of 2 corresponds to a reduction of the effective length to  $1/\sqrt{2}$  or .707 times the total length.

**Short Columns:** If the length of a column is reduced below a certain critical value, primary bending failure will occur at loads below those predicted by the Euler formula. This is due to a reduction in the effective value of  $E$  which is caused by changes in the slope of the stress-strain diagram and by unavoidable eccentricities. In this region the test results show more scatter than in the Euler range and it is customary to

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adopt an empirical or semi-empirical formula for predicting the allowable column stress. When a definite eccentricity exists, the critical column loads are reduced due to the combined effects of axial load and bending. Special formulas for such cases can be found in standard textbooks and handbooks.

Although many types of formulas have been devised to cover the short-column range, it has been customary, in aircraft work, to use the Johnson formula for round steel tubes and the straight line formula for round aluminum alloy tubes. Recent tests at the National Bureau of Standards have shown, however, that the Johnson formula does not have the correct shape for round tubes of normalized X-4130 steel.

A modified parabola having an exponent 1.5 (Eq. 3:18) gives a satisfactory representation of the Bureau of Standards test data on round tubes of this material and it has therefore been adopted for this handbook. The Johnson formula (Eq. 3:17) will be used for round 1025 steel tubes and round heat-treated alloy steel tubes.

It will be noted that the above column formulas are of the general form given by Eq. 3:16. For example, the straight line formula is a special case of Eq. 3:16 in which the exponent  $n$  is equal to 1.0. In a similar manner the Johnson formula is obtained by setting  $n$  equal to 2.0. The above equations strictly apply only to round tube sections as they were derived from tests on such sections. In many cases, however, they will be found to be satisfactory for selections of other shapes when local instability is not critical.

**Column Yield Stress ( $F_{co}$ ):** The upper limit of the allowable column stress for primary failure is called the *column yield stress* and will be designated  $F_{co}$ . It can be determined by extending the "short-column" curve to a point corresponding to zero length, neglecting any tendency of the curve to rise rapidly or "pick-up" for very short lengths. The short-column curve used in determining  $F_{co}$  should be obtained from tests on specimens having geometrical proportions such that local failure is precluded except for very low values of  $L'/\rho$ .

When the column yield stress is reached, the walls of the column will tend to buckle unless restrained by extreme shortness, or by the application of lateral restraining forces. In some cases, however, if the specimen has not been allowed to buckle, the stress may be increased considerably above this value. Due to the danger of buckling when the column yield stress is approached, the latter should be considered as the limiting stress for all columns.

The column yield stress is mainly determined by the nature of the compressive stress-strain diagram of the material. When the material has a definite yield point in compression, this value may be assumed for the column yield stress. Few aircraft materials, however, have a sharply defined yield point. In such cases it is usually possible to determine the column yield stress as a function of either the tensile or compressive yield stress. For example,  $F_{co}$  for normalized X-4130 round tubes is approximately equal to 1.06 times the tensile yield stress.

### 5. NON-DIMENSIONAL COLUMN CURVES FOR PRIMARY FAILURE

**General:** On account of the many factors involved it is often difficult to predict the effects of possible material variations on the strength of columns as obtained by tests. When the column failure is definitely of the *primary bending type* it is

advisable to plot the test results with non-dimensional coefficients. The following coefficients will be adopted for this purpose.

$$3:27 \quad R_a = \text{allowable stress ratio} \\ = F_c / F_{co} \\ \text{where } F_c = \text{allowable column stress} \\ F_{co} = \text{column yield stress}$$

$$3:28 \quad B = \text{slenderness ratio factor} \\ = \frac{L'/\rho}{\pi \sqrt{E/F_{co}}}$$

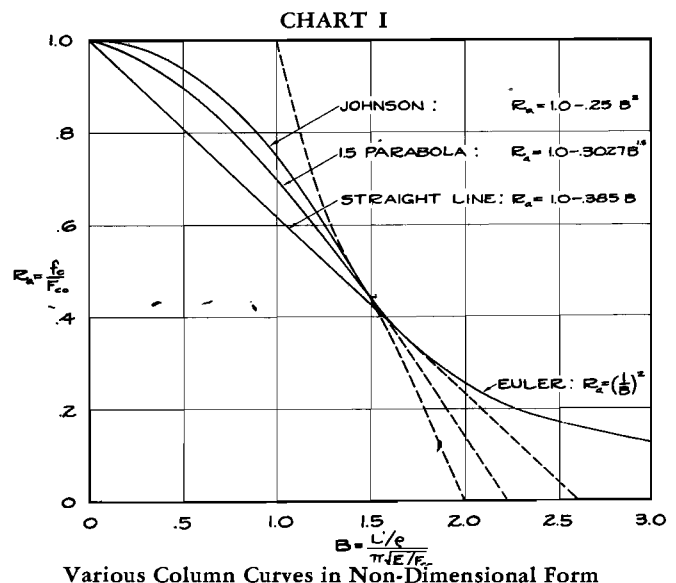
$$L' = L/\sqrt{c} \quad (\text{See eq. 3:15})$$

The slenderness ratio factor can be considered as the ratio between the effective slenderness ratio ( $L'/\rho$ ) and the ( $L'/\rho$ ) at which the Euler stress for a pin-ended column would equal  $F_{co}$ . Thus, when  $B = 2$ , the Euler stress  $F_{ce}$  would equal  $\frac{1}{4} F_{co}$ , or  $R_a$  would be .25 (since the Euler stress varies inversely as the square of  $L'/\rho$ ).

**Typical Column Curves:** Typical column curves plotted in terms of these non-dimensional coefficients are illustrated in Chart I. It will be noted that the Johnson parabolic curve is tangent to the Euler curve at a value of  $R_a = .5$ ; that is, the Euler formula will not apply when it gives stresses higher than half the column yield stress. It is also convenient to know that the stresses given by the 1.5 parabolic formula and the straight line formula are equal to those given by the Euler formula at values of  $R_a$  equal to .4286 and .333 respectively.

**Charts of Strength of Steel Tubes:** The formulas used in computing these charts are given in Table 7. The symbols used are defined in Part 1 of this section.

**Effects of Welding:** The primary failure stress of a column having welded ends is determined from the formulas of Table 7 without regard for the effects of welding. These stresses, however, should not exceed a "cut-off" stress which accounts for the effects of welding on the local failure of the column. In the case of X-4130 tubing having tensile yield stresses of 75,000 and 85,000 psi, the welding cut-offs are at stresses of 67,500 and 76,600 psi, respectively.



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**TABLE 7**  
**COLUMN FORMULAS FOR STEEL TUBES**

Tubes	Material Steel	Fty—psi	Fco—psi	Short Column Formulas(a)	Critical <sup>(b)</sup> $L'/\rho$	Long Column Formulas
	1025	36,000	36,000	$36,000 - 1.172 \left(\frac{L'}{\rho}\right)^2$	124	$276 \times 10^6 \left(\frac{L'}{\rho}\right)^2$
Round and Square	X-4130	75,000	79,500	$79,500 - 51.9 \left(\frac{L'}{\rho}\right)^{1.5}$	91.5	$286 \times 10^6 \left(\frac{L'}{\rho}\right)^2$
	X-4130	85,000	90,100	$90,100 - 64.4 \left(\frac{L'}{\rho}\right)^{1.5}$	86.0	$286 \times 10^6 \left(\frac{L'}{\rho}\right)^2$
				See footnote (c)		
	X-4130	75,000	79,500	$79,500 - K \left(\frac{L'}{\rho}\right)^{1.5}$	See footnote (c)	$286 \times 10^6 \left(\frac{L'}{\rho}\right)^2$
Stream-line and Elliptical				See footnote (c)		
	X-4130	85,000	90,100	$90,100 - K \left(\frac{L'}{\rho}\right)^{1.5}$	See footnote (c)	$286 \times 10^6 \left(\frac{L'}{\rho}\right)^2$

Note (a)  $\frac{L'}{\rho} = \frac{L}{\rho \sqrt{c}}$

Note (b) Critical  $\frac{L'}{\rho}$  is that above which columns are "long" and below which they are "short."

Note (c) The values of K and critical  $\frac{L'}{\rho}$  are functions of  $\frac{R}{t}$  of the tube. See charts Nos. 2, 3, 4 and 5 for these values.

## 6. FAILURE UNDER COMBINED LOADING

For combined loading conditions in which failure is caused by buckling or instability, no general theory exists which will apply in all cases. It is convenient, however, to represent such conditions by the use of "stress ratios," which can be considered as non-dimensional coefficients denoting the fraction of the allowable stress or strength which is utilized or which can be developed under special conditions. For simple stresses the stress ratio can be expressed as:

$$3:29 \quad R = \frac{f}{F}$$

where  $f$  = applied stress  
 $F$  = allowable stress

Note that the "margin of safety" as usually expressed, is given by the equation:

$$3:30 \quad \text{M.S.} = \frac{1}{R} - 1.0$$

Considering the case of combined loadings, the general conditions for failure can be expressed by equations of the following type:

$$3:31 \quad R_1^x + R_2^y + R_3^z + \dots = 1.0$$

In this equation  $R_1$ ,  $R_2$  and  $R_3$  may denote, for instance, the stress ratios for compression, bending, and shear, and the exponents  $x$ ,  $y$  and  $z$  define the general relationship of the quantities. This equation may be interpreted as indicating that failure will occur only when the sum of the stress ratios is equal to or greater than one. An advantage of this method is that the formula yields correct results when only one loading condition is present. Consequently it tends to give good results when any one loading condition predominates. It also permits test data to be plotted in non-dimensional form, which is a decided advantage.

In many cases it is convenient to deal directly with "load ratios" rather than stress ratios. The load ratio is simply the ratio of the applied load to the allowable load and is equal to the corresponding stress ratio.

Considering only two loading conditions, such as bending and torsion, Eq. 3:31 can be plotted as a single interaction curve of  $R_b$  against  $R_s$ . Likewise, in the case of combined bending and compression,  $R_c$  can be plotted against  $R_b$ . When all three conditions exist, the equation represents an interaction surface, which can be plotted as a family of curves. Typical curves corresponding to various exponents are shown in Chart 6. The general significance of Eq. 3:31 and the chart just mentioned is that the addition of a second loading condition will lower the percentage of the allowable stress which may be utilized in the original loading condition. If the exponents approach infinity, the curve of Chart 6 will approach the lines  $R_1 = 1.0$  and  $R_2 = 1.0$ , indicating that the two loading conditions have no effect on each other.

When only two stress ratios are involved and when the two different applied stresses remain in constant proportion, the margin of safety of the member may be determined from Chart 6 by the following method:

- (1) Locate the point on the chart representing the applied values of  $R_1$  and  $R_2$  computed from the applied stresses. [Illustrated as point (1)].
- (2) Draw a straight line through this point and the origin (shown as a diagonal dotted line in the figure).
- (3) Extend this line to intersect the proper stress-ratio curve (corresponding to the condition under consideration) at point (2).
- (4) Read the allowable values  $R_{1a}$  and  $R_{2a}$  as the ordinate and abscissa, respectively, of point (2).
- (5) The factor of utilization or strength ratio is obtained as the ratio of the applied to the allowable value of either stress ratio as follows:

$$2:32 \quad U = \frac{R_1}{R_{1a}} = \frac{R_2}{R_{2a}}$$

- (6) The true margin of safety then can be computed from the following equation:

$$2:33 \quad \text{M.S.} = \frac{1}{U} - 1$$

Note that when the following stress ratio expressions are used, the margins of safety can be computed as indicated.

$$\text{For } R_1 + R_2 = 1$$

$$\text{M.S.} = \frac{1}{R_1 + R_2} - 1$$

$$\text{For } R_1^2 + R_2^2 = 1$$

$$\text{M.S.} = \frac{1}{\sqrt{R_1^2 + R_2^2}} - 1$$

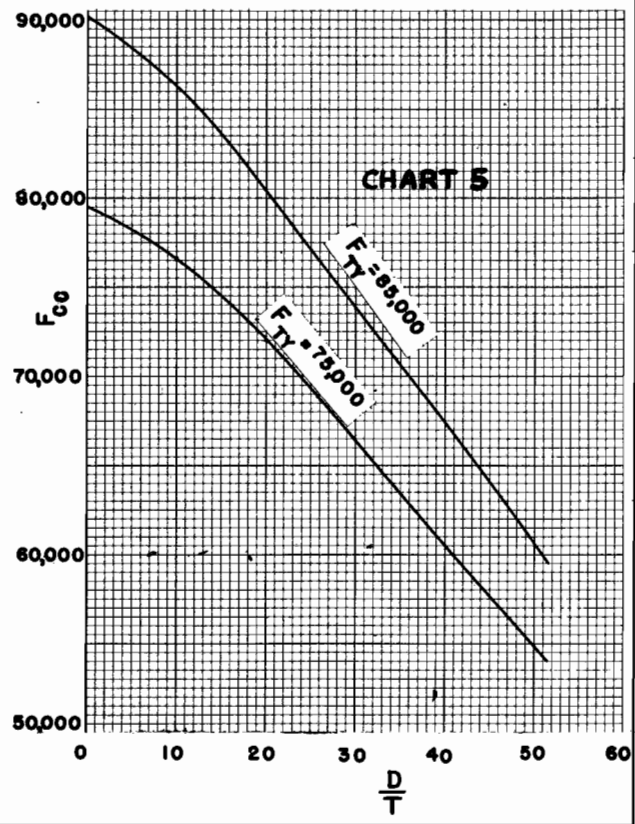
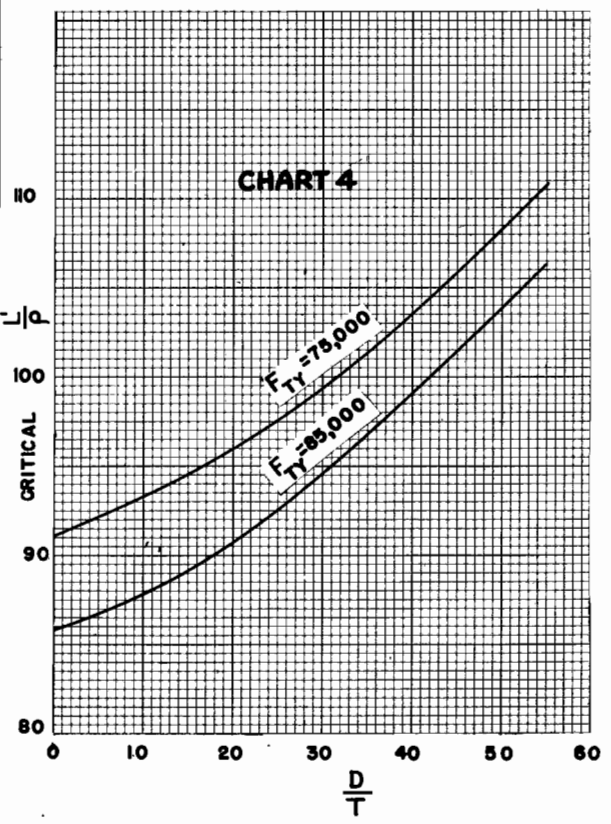
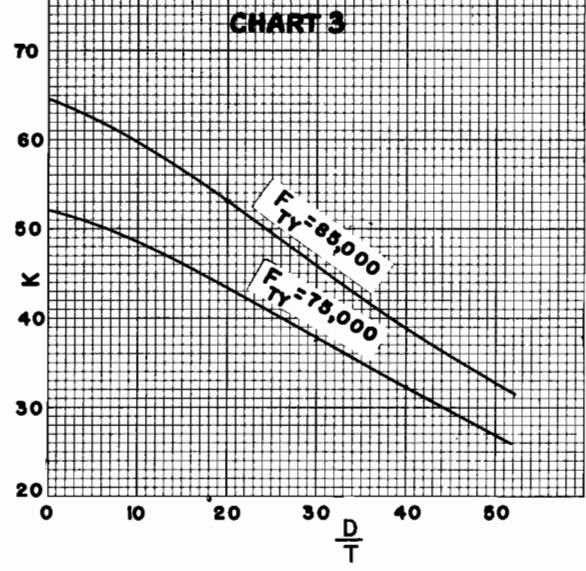
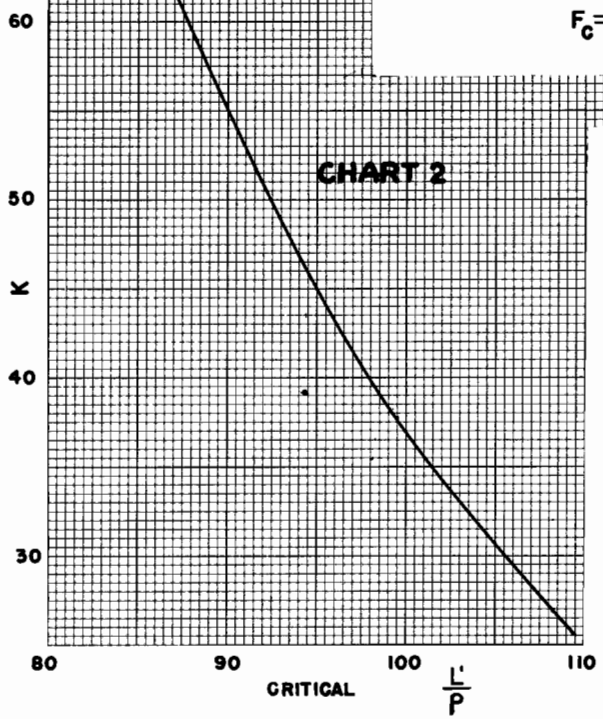
Other M.S. formulas can, of course, be determined for the more complicated stress ratio expressions.



**STREAMLINED AND ELLIPTICAL TUBES**  
DATA FOR COLUMN FORMULAS

FORMULA  

$$F_c = \frac{P}{A} = F_{CC} - K \left( \frac{L}{P} \right)^{1.5}$$



## DESIGN DATA AND FORMULAE

### 7. EXAMPLES OF USE OF COLUMN CHARTS

The column charts give the allowable column loads of tubes as a function of their lengths and end fixity,  $c = 1$  or  $c = 2$ . For a required load to be carried and a given length, it is a simple matter to select the lightest tube capable of meeting the requirements.

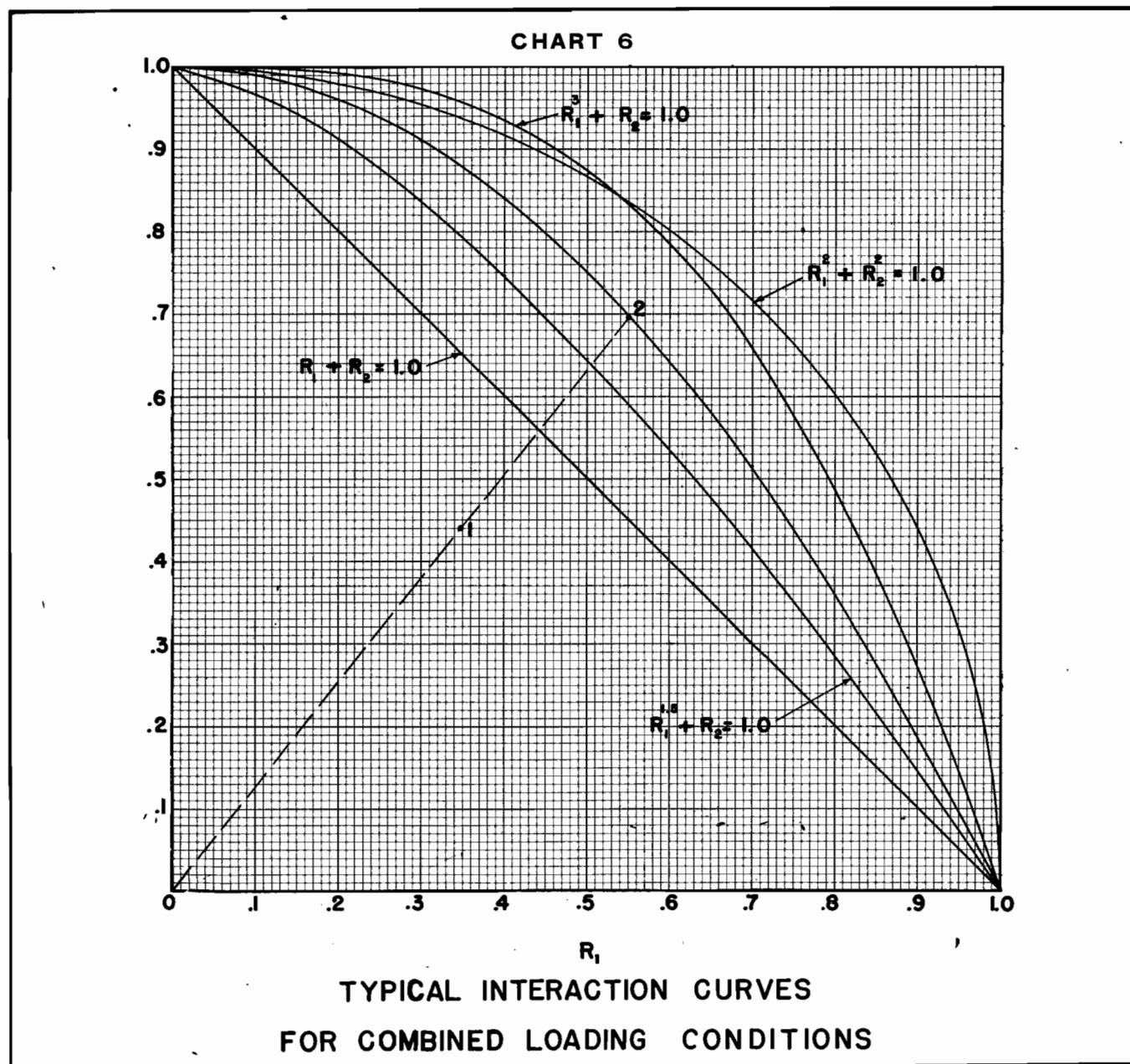
For example, if a round tube, 50 inches long, welded at the ends ( $c = 2$ ) is required to carry a load of 28,000 pounds, select the lightest tube available. Assume the material to be special X-4130,  $F_{tu} = 100,000$ ,  $F_{ty} = 85,000$ . Tubes on the following two adjacent charts will satisfy the requirements:

ROUNDS, 85,000,  $D = 1\frac{5}{8}"$  to  $1\frac{7}{8}"$ , and

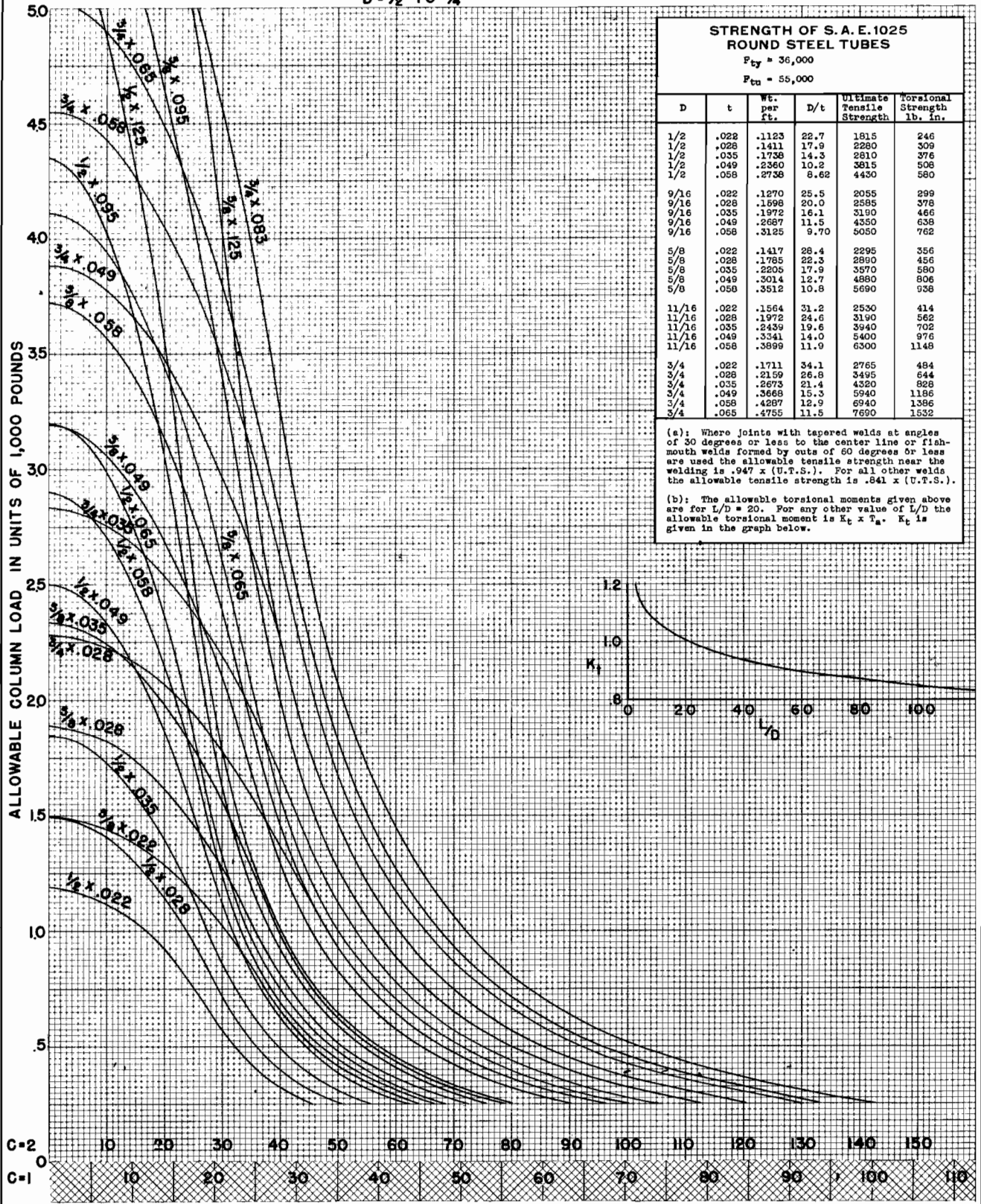
ROUNDS, 85,000,  $D = 2"$  to  $2\frac{1}{2}"$ .

From the first we find that of the two probable tubes  $1\frac{7}{8}" \times 0.083"$  and  $1\frac{3}{4}" \times 0.095"$  the first is the lighter. From the second we find that the  $2\frac{1}{4}" \times 0.058"$  is the lightest. We note in the charts in the upper right-hand corners that the weight of the  $1\frac{7}{8}" \times 0.083"$  tube is 1.589 pounds per foot and the weight of the  $2\frac{1}{4}" \times 0.058"$  tube is 1.358 pounds per foot. Thus if the lightness is the criterion the  $2\frac{1}{4}" \times 0.058"$  tube would be selected. For convenience of the engineer, the tensile strength, the bending strength, and the torsional strength of the tubes are repeated on the charts.

It should be particularly noted that in selecting a tube for lightness adjacent charts should be consulted for possible lighter tubes.



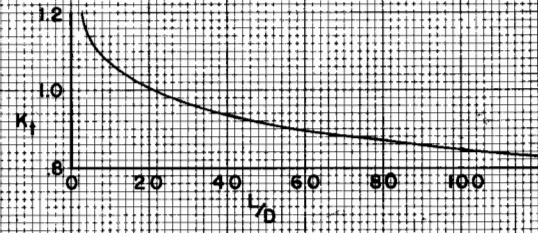
$D = \frac{1}{2}'' \text{ TO } \frac{3}{4}''$

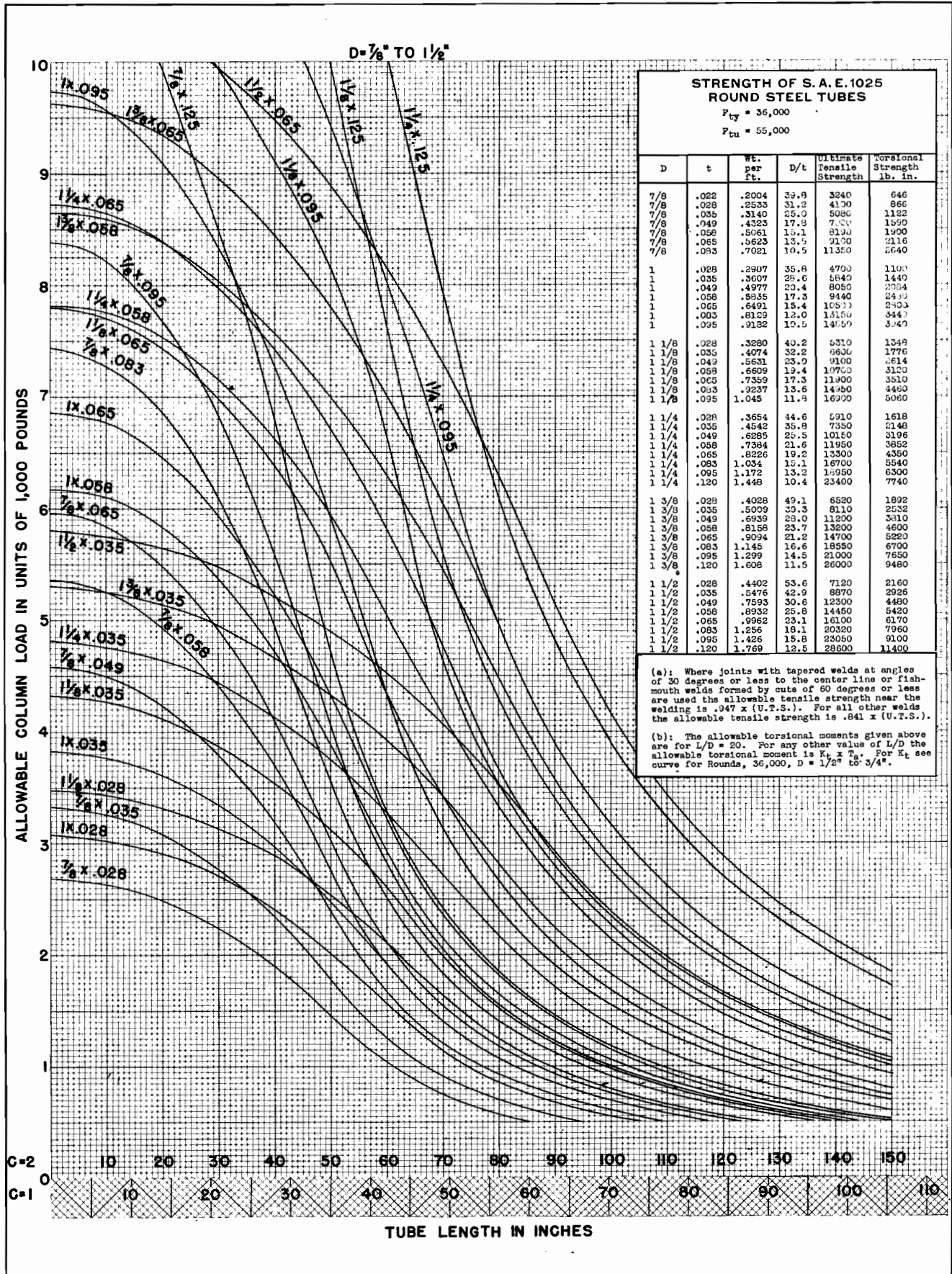


**STRENGTH OF S. A. E. 1025 ROUND STEEL TUBES**  
 $F_{ty} = 36,000$   
 $F_{tu} = 55,000$

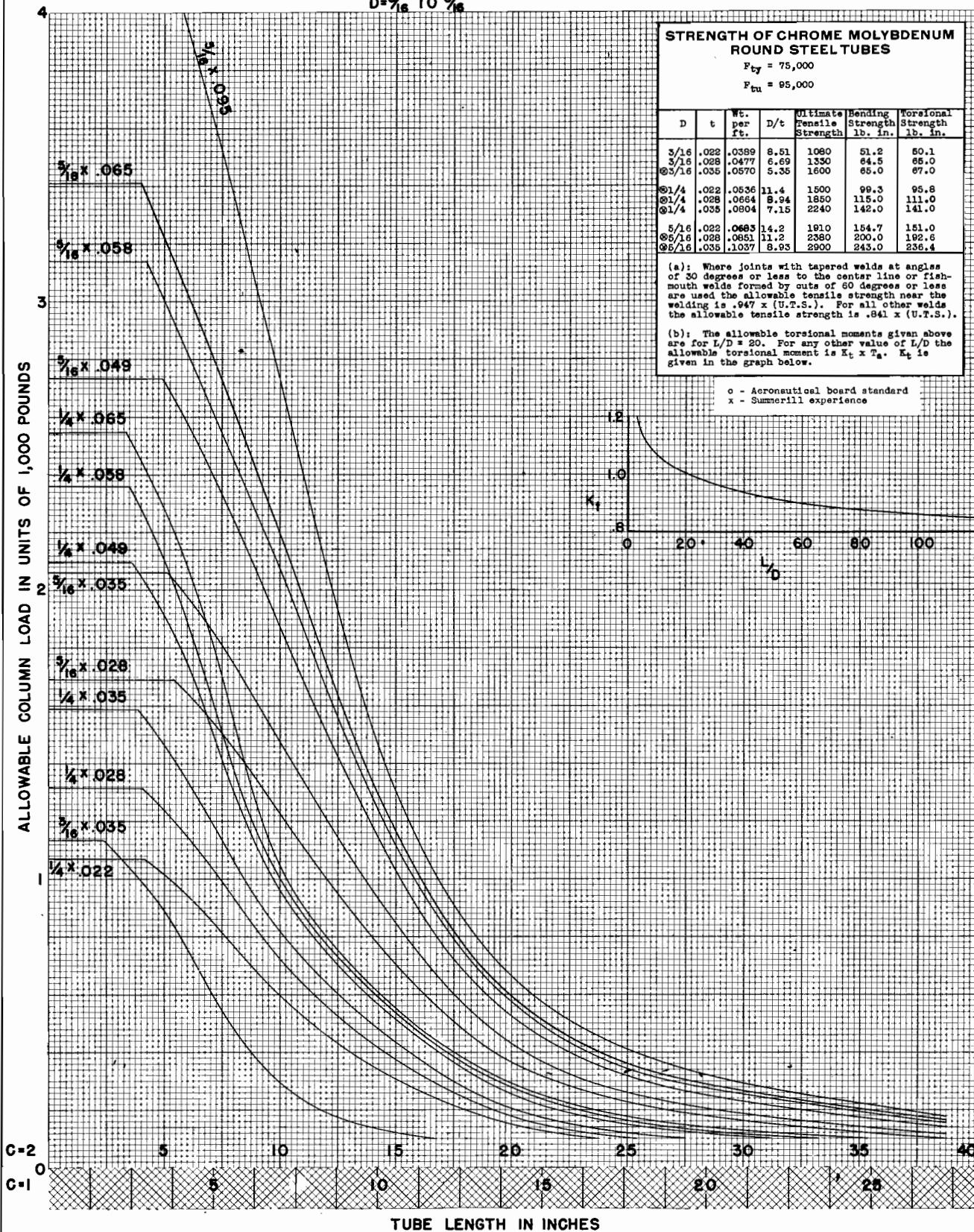
D	t	Wt. per ft.	D/t	Ultimate Tensile Strength	Torsional Strength lb. in.
1/2	.022	.1123	22.7	1815	246
1/2	.028	.1411	17.9	2280	309
1/2	.035	.1738	14.3	2810	376
1/2	.049	.2360	10.2	3615	508
1/2	.058	.2738	8.62	4430	580
9/16	.022	.1270	25.5	2055	299
9/16	.028	.1598	20.0	2585	378
9/16	.035	.1972	16.1	3190	466
9/16	.049	.2687	11.5	4350	638
9/16	.058	.3125	9.70	5050	762
5/8	.022	.1417	28.4	2295	356
5/8	.028	.1785	22.3	2890	456
5/8	.035	.2205	17.9	3570	580
5/8	.049	.3014	12.7	4880	806
5/8	.058	.3512	10.8	5690	938
11/16	.022	.1564	31.2	2530	414
11/16	.028	.1972	24.6	3190	562
11/16	.035	.2439	19.6	3940	702
11/16	.049	.3341	14.0	5400	976
11/16	.058	.3899	11.9	6300	1148
3/4	.022	.1711	34.1	2765	484
3/4	.028	.2159	26.8	3495	644
3/4	.035	.2673	21.4	4320	828
3/4	.049	.3668	15.3	5940	1186
3/4	.058	.4287	12.9	6940	1386
3/4	.065	.4755	11.5	7960	1532

(a): Where joints with tapered welds at angles of 30 degrees or less to the center line or fish-mouth welds formed by cuts of 60 degrees or less are used the allowable tensile strength near the welding is .947 x (U.T.S.). For all other welds the allowable tensile strength is .841 x (U.T.S.).  
 (b): The allowable torsional moments given above are for  $L/D = 20$ . For any other value of  $L/D$  the allowable torsional moment is  $K_t \times T_a$ .  $K_t$  is given in the graph below.





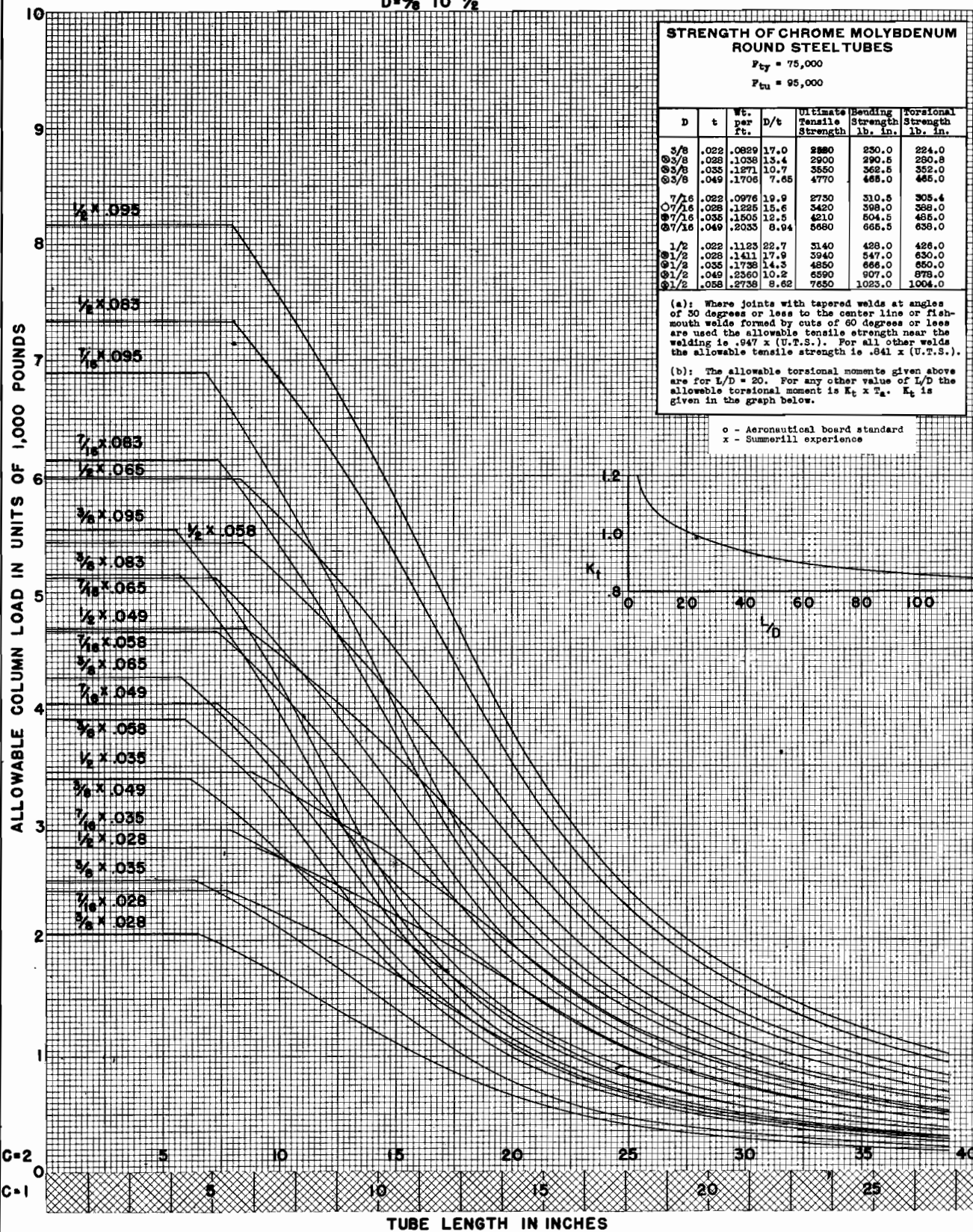
$D = \frac{3}{16}'' \text{ TO } \frac{5}{16}''$



For Rounds — 85,000 Yield See III — 42

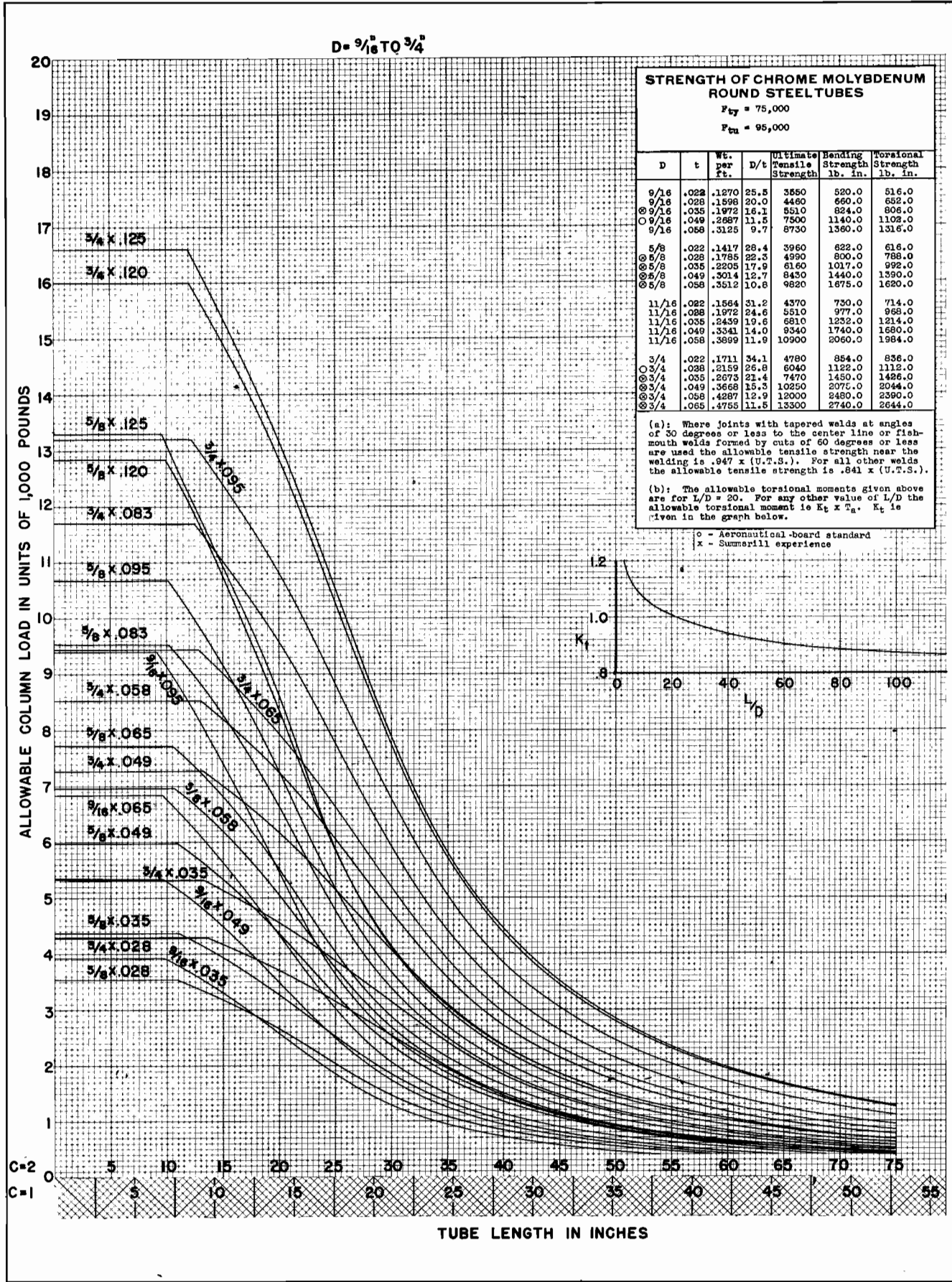


D = 3/8" TO 1/2"



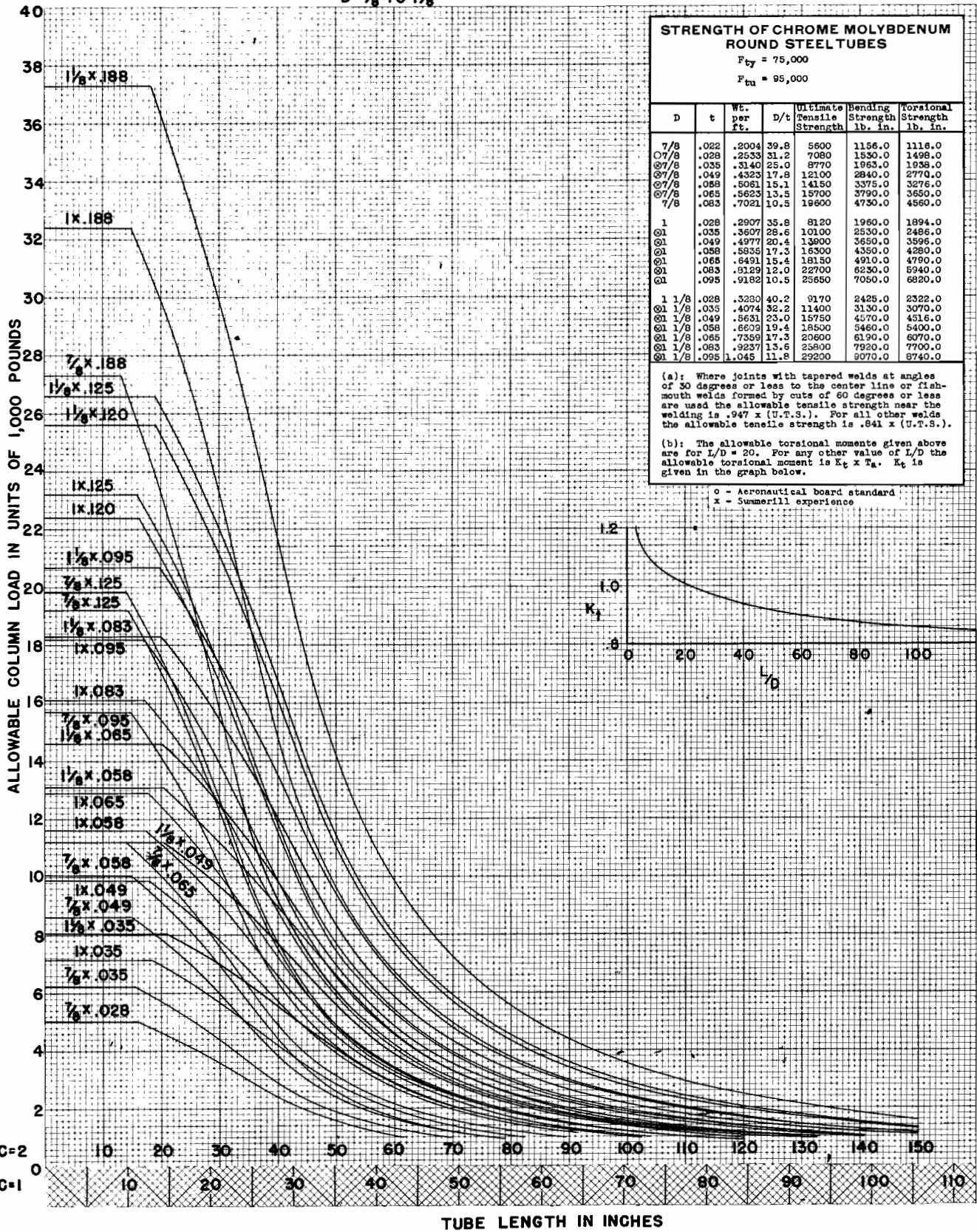
For Rounds — 85,000 Yield See III — 42

$$D = \frac{3}{16} \text{ TO } \frac{3}{4}$$



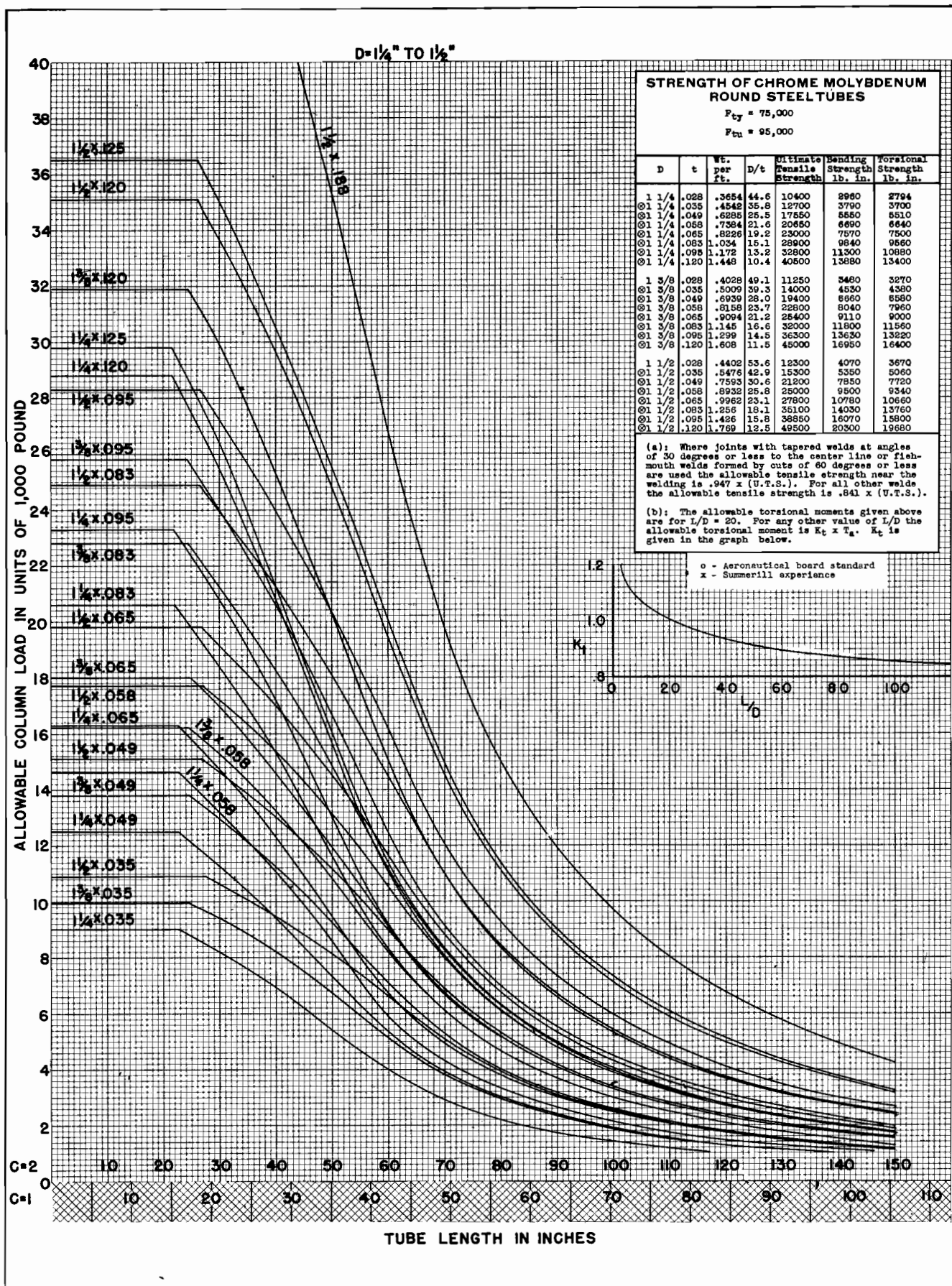
For Rounds — 85,000 Yield . See III — 42

D = 7/8" TO 1 1/8"



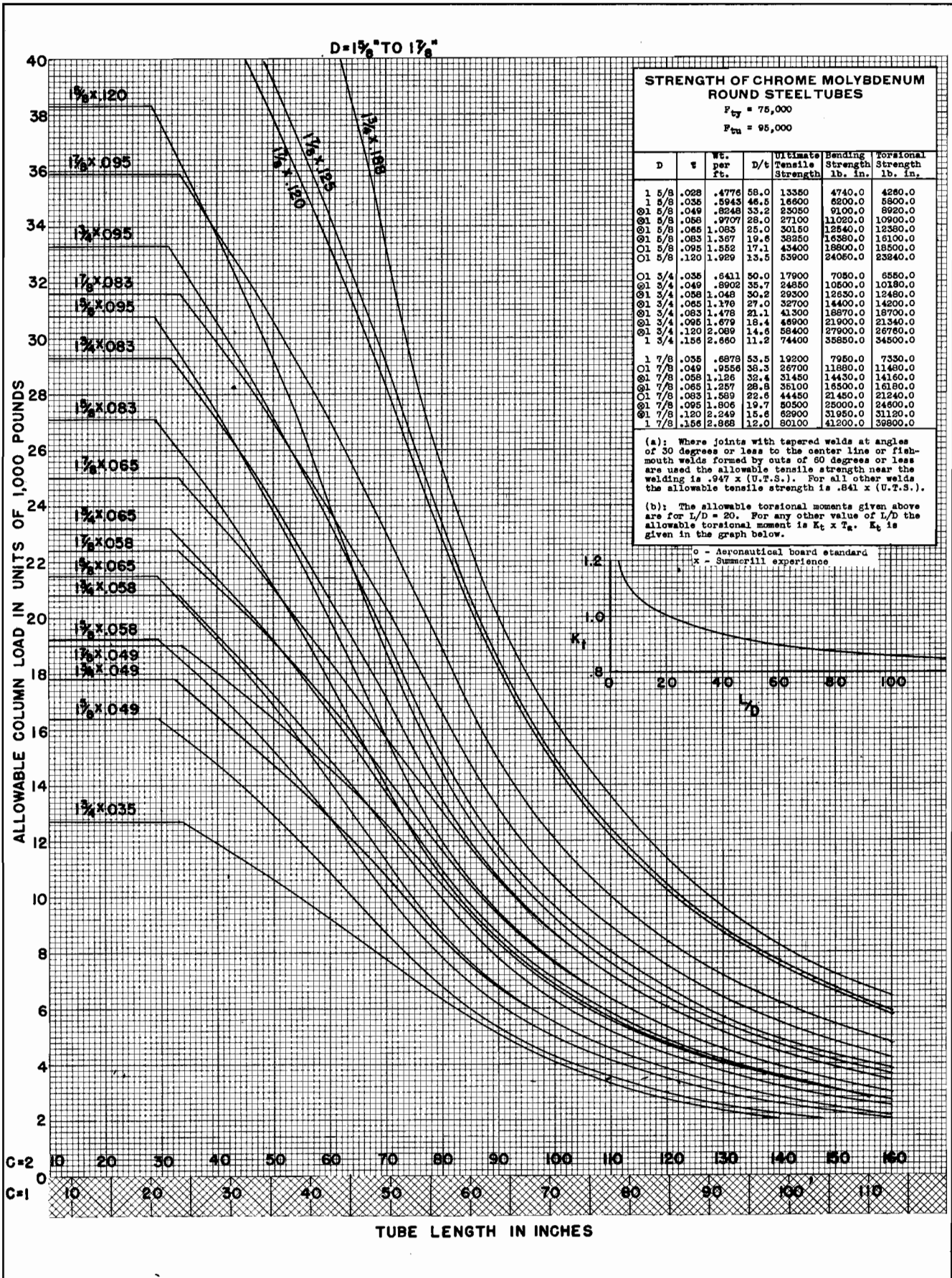
For Rounds — 85,000 Yield See III — 42



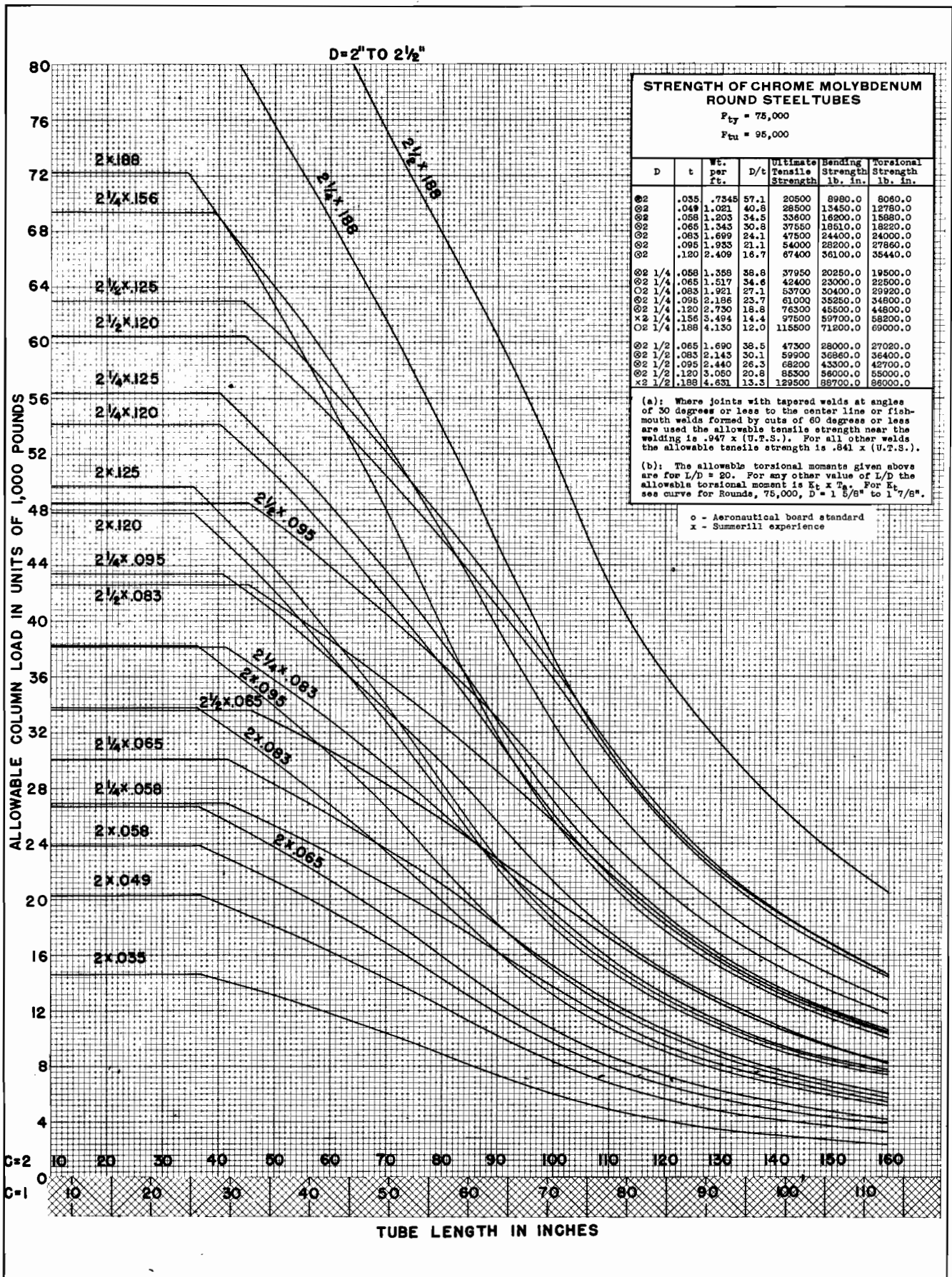


For Rounds — 85,000 Yield - See III — 42

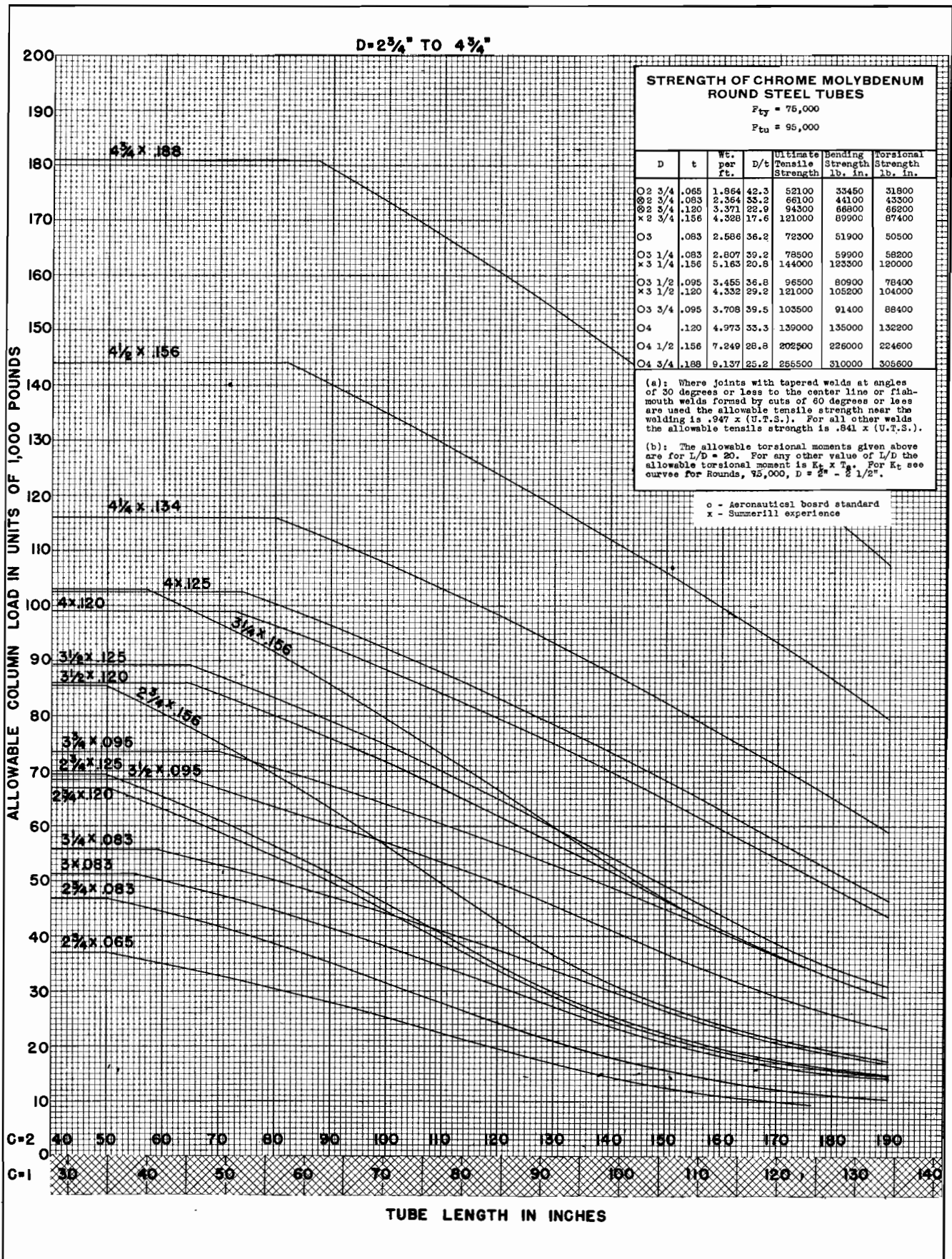




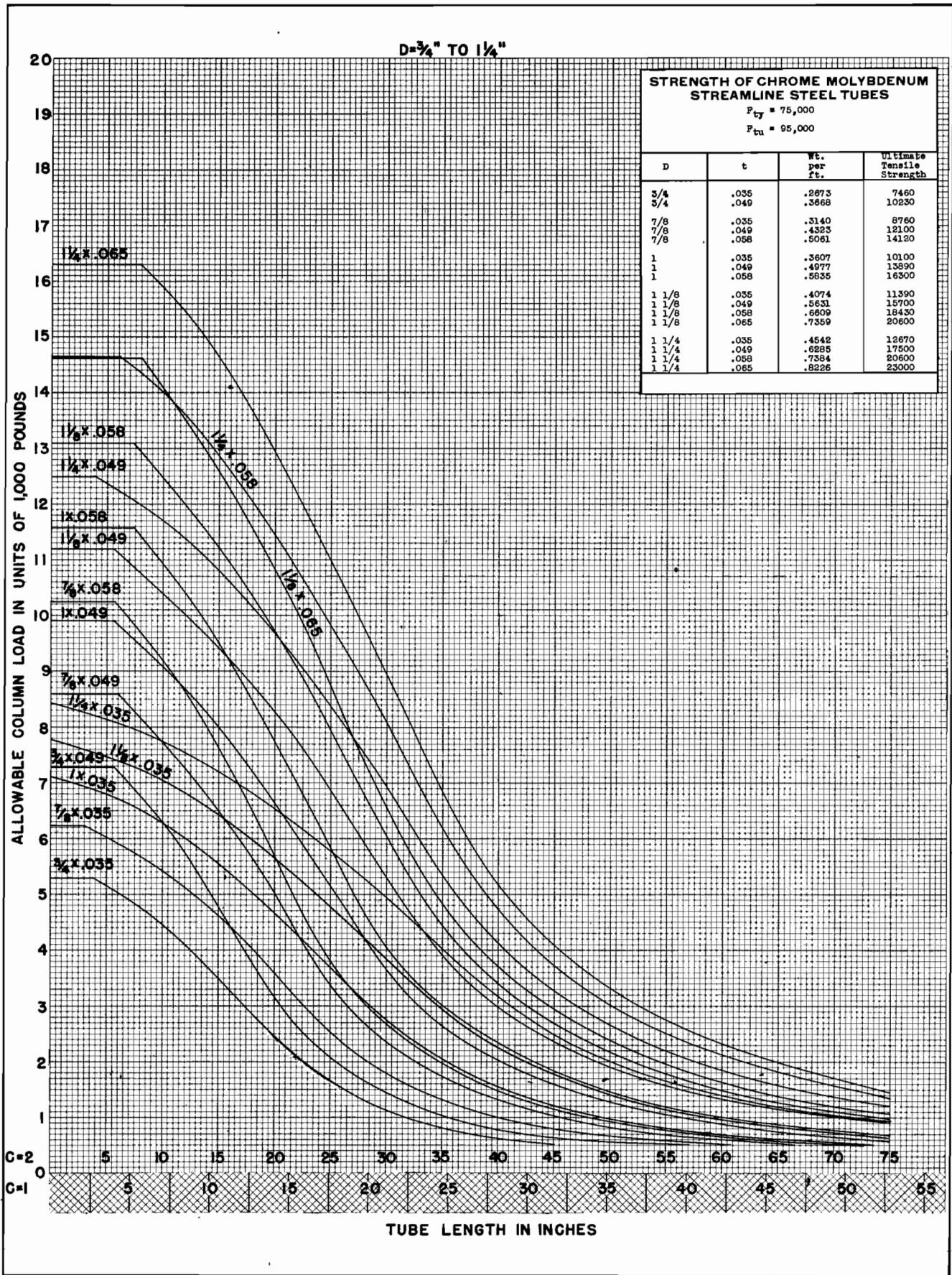
For Rounds — 85,000 Yield . See III — 42



For Rounds — 85,000 Yield - See III — 42

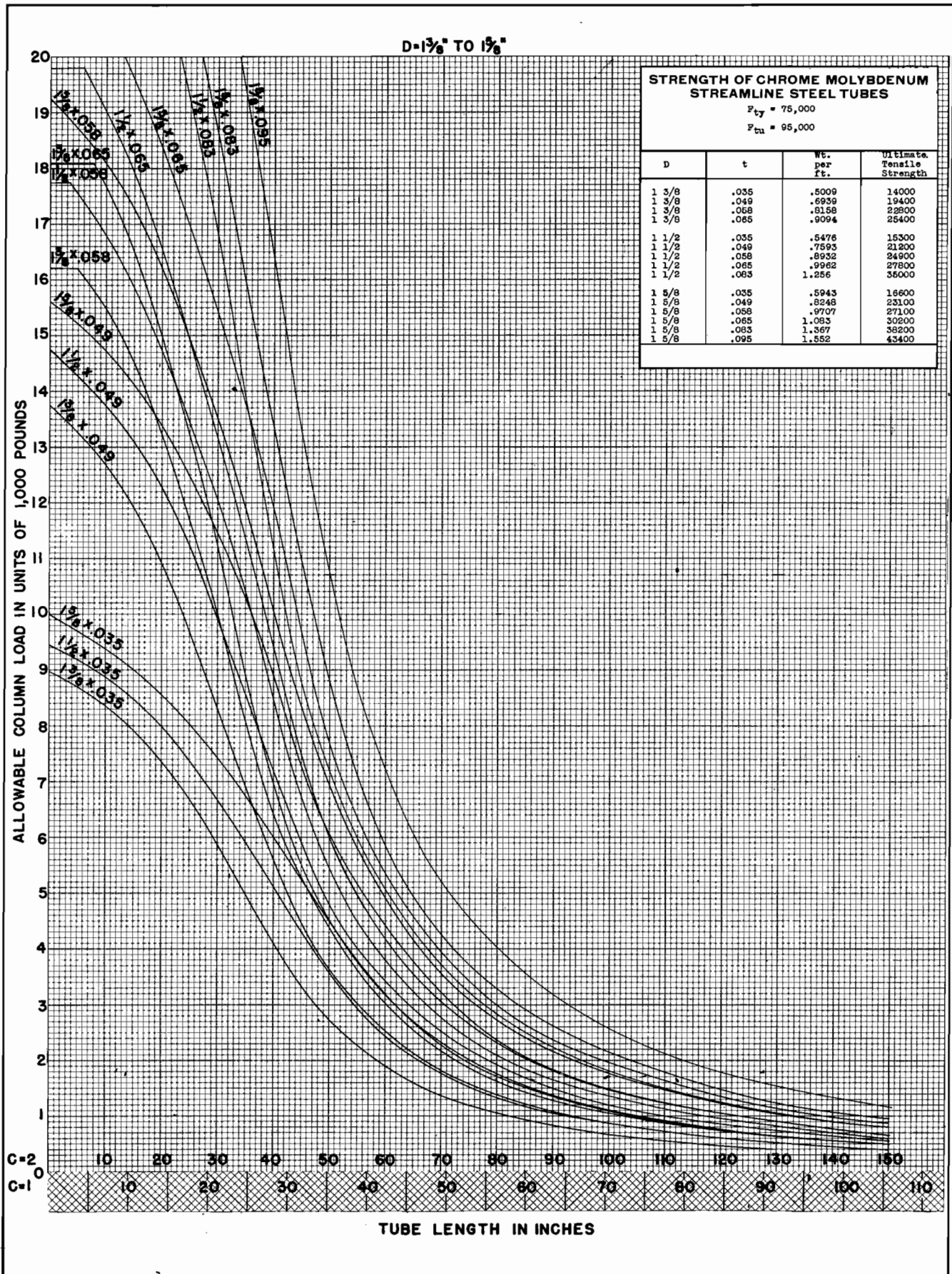


For Rounds — 85,000 Yield See III — 42

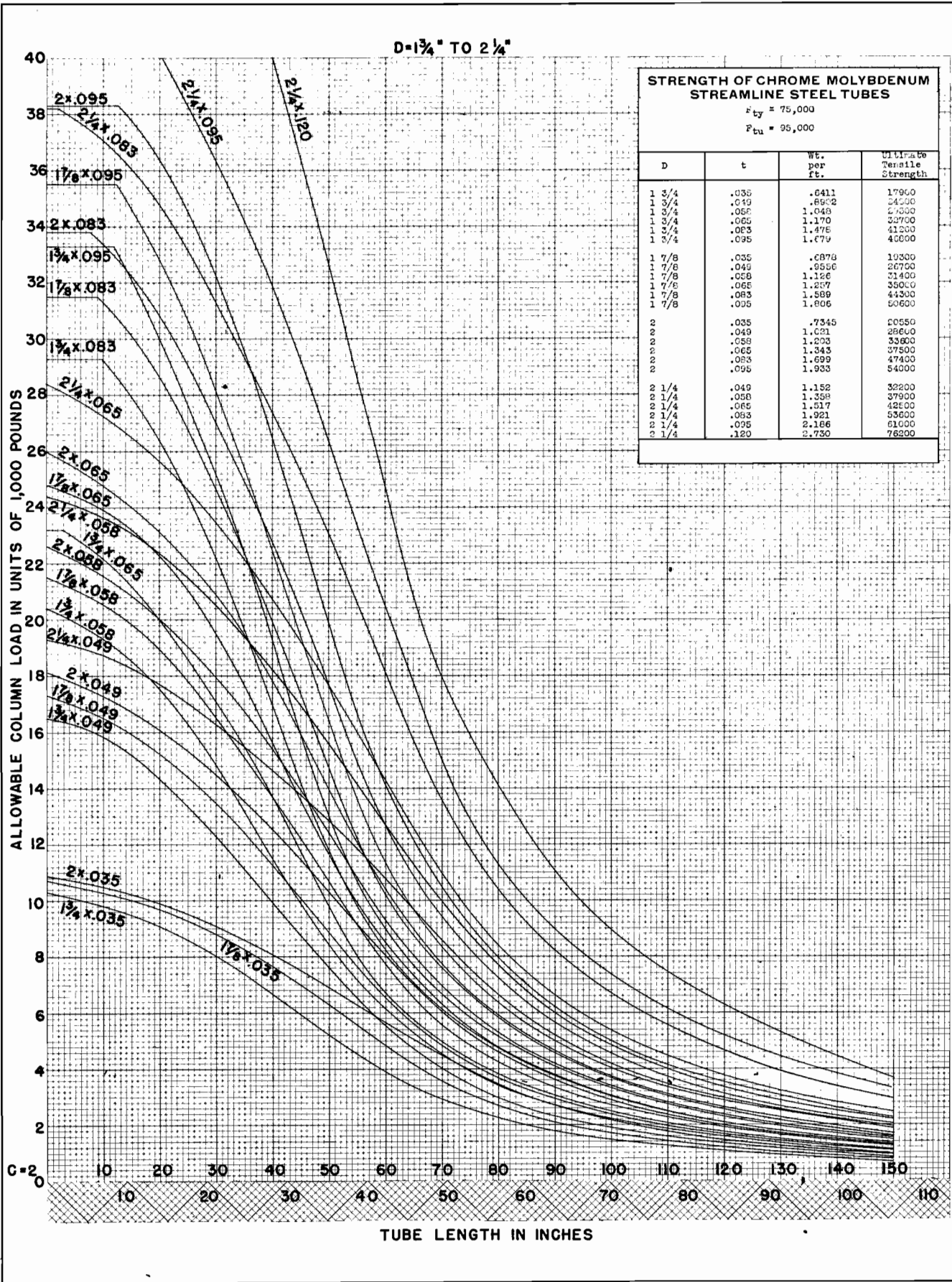


For Streamline — 85,000 Yield See III — 50





For Streamline — 85,000 Yield. See III — 50



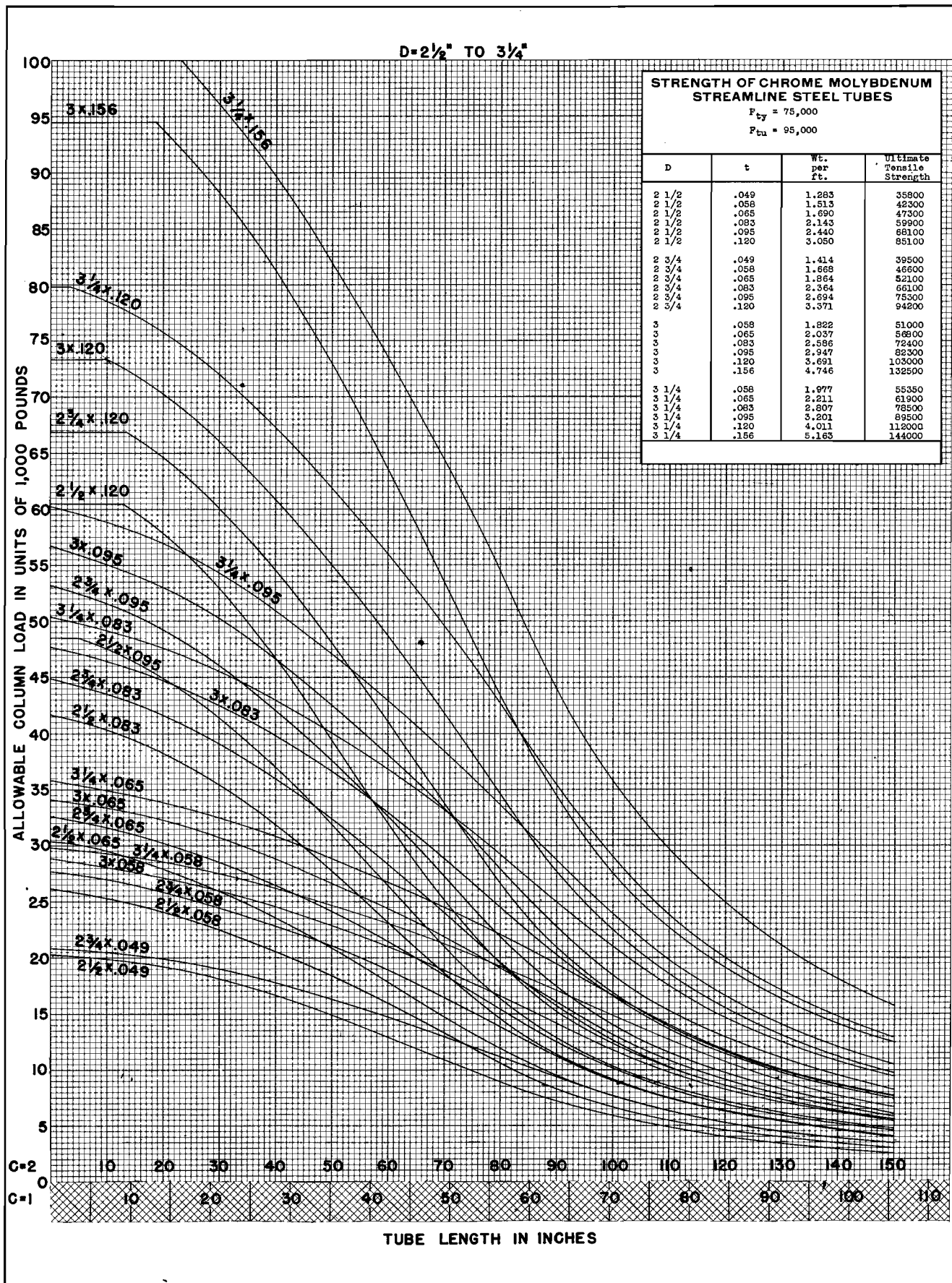
**STRENGTH OF CHROME MOLYBDENUM  
STREAMLINE STEEL TUBES**

$F_{by} = 75,000$   
 $F_{bu} = 95,000$

D	t	Wt. per ft.	Ultimate Tensile Strength
1 3/4	.035	.6411	17900
1 3/4	.049	.8932	24500
1 3/4	.065	1.048	27500
1 3/4	.083	1.170	32700
1 3/4	.095	1.278	36000
1 7/8	.035	.6878	19500
1 7/8	.049	.9556	26700
1 7/8	.065	1.126	31400
1 7/8	.083	1.287	35000
1 7/8	.095	1.389	38500
2	.035	.7345	20550
2	.049	1.021	28600
2	.065	1.233	33600
2	.083	1.345	37500
2	.095	1.439	41400
2 1/4	.049	1.152	32000
2 1/4	.065	1.517	42500
2 1/4	.083	1.921	53600
2 1/4	.095	2.166	61000
2 1/4	.120	2.730	76200

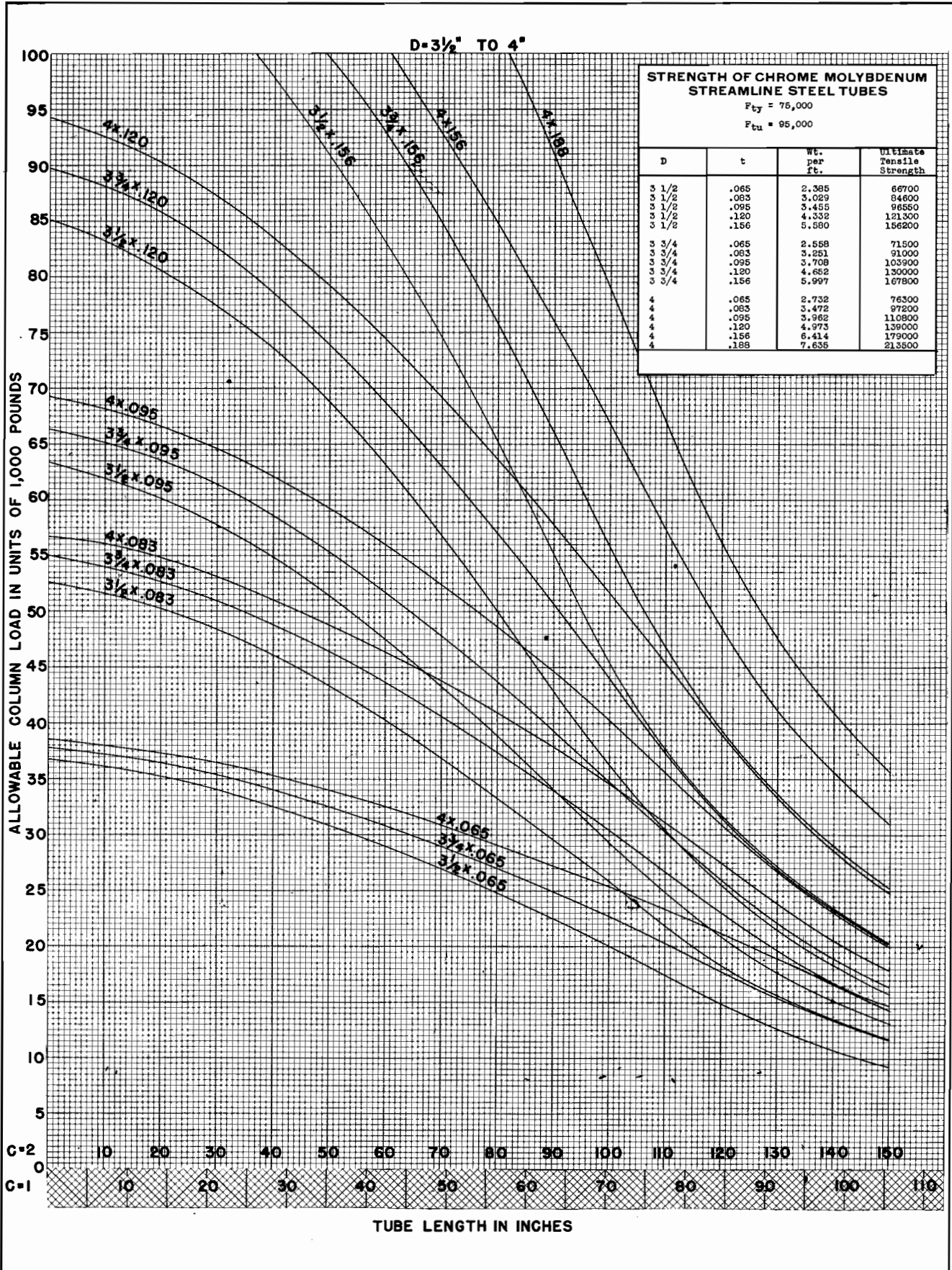
For Streamline — 85,000 Yield See III — 50





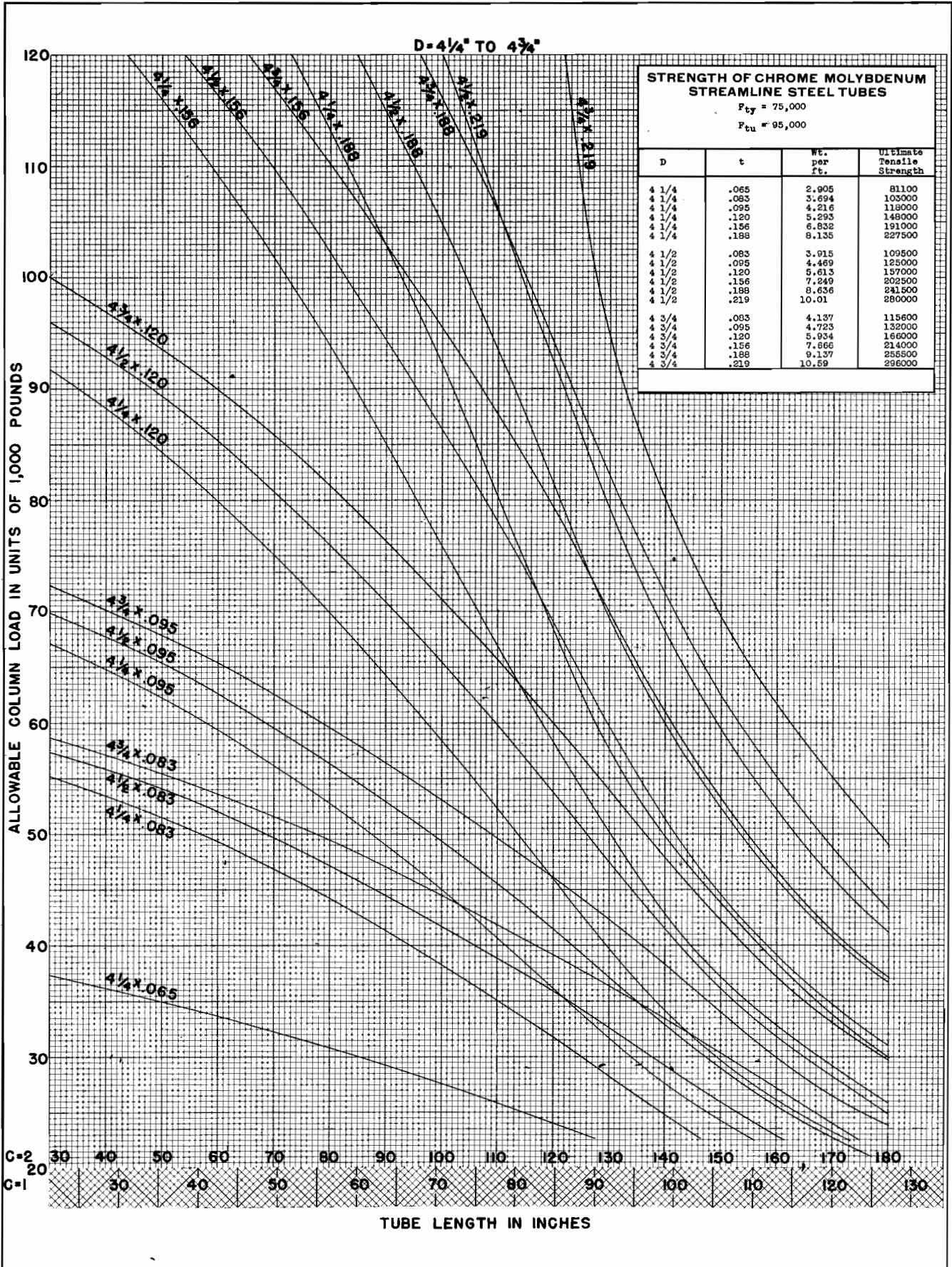
For Streamline — 85,000 Yield . See III — 50





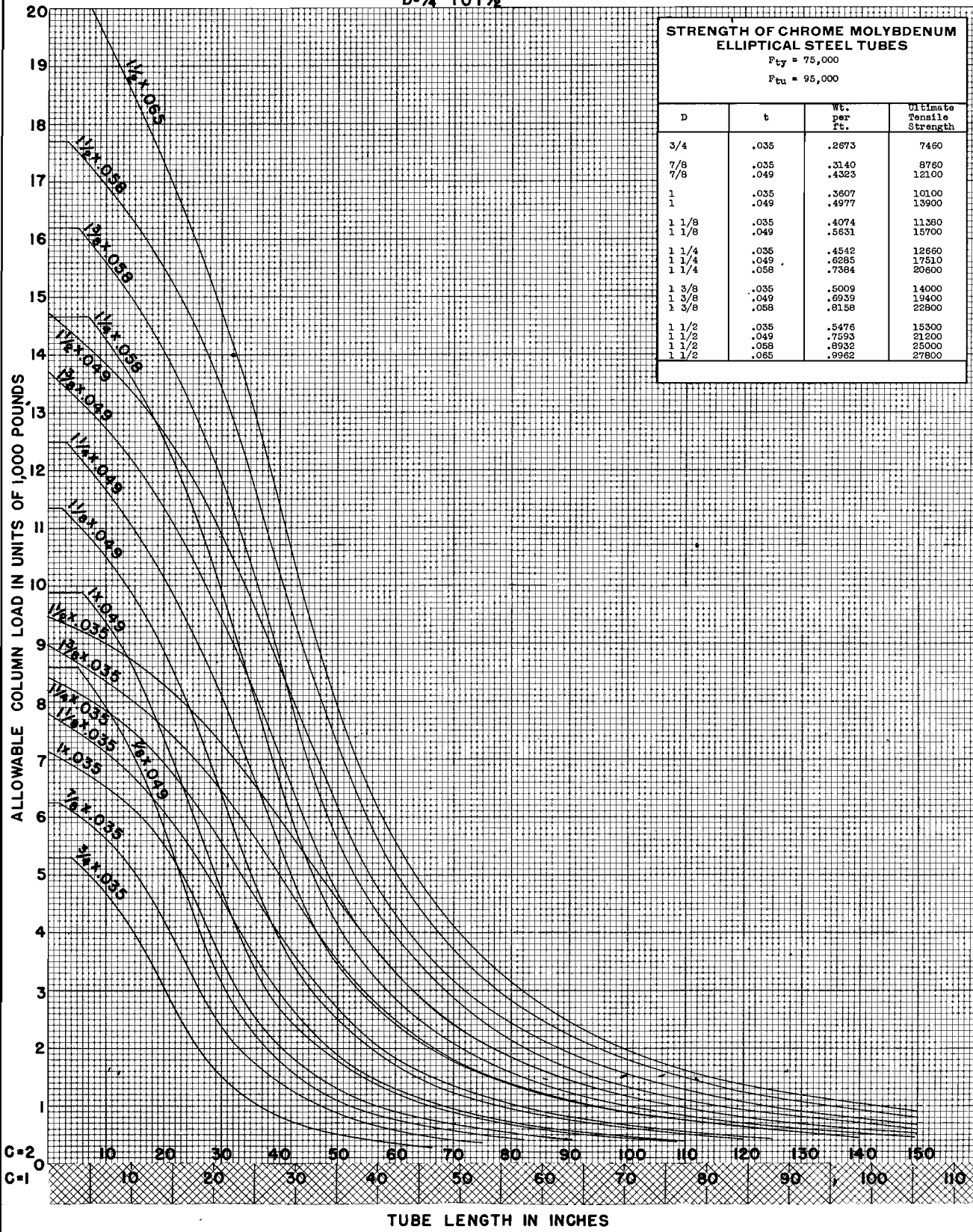
For Streamline — 85,000 Yield See III — 50





For Streamline — 85,000 Yield See III — 50

$D = \frac{3}{4}''$  TO  $1\frac{1}{2}''$



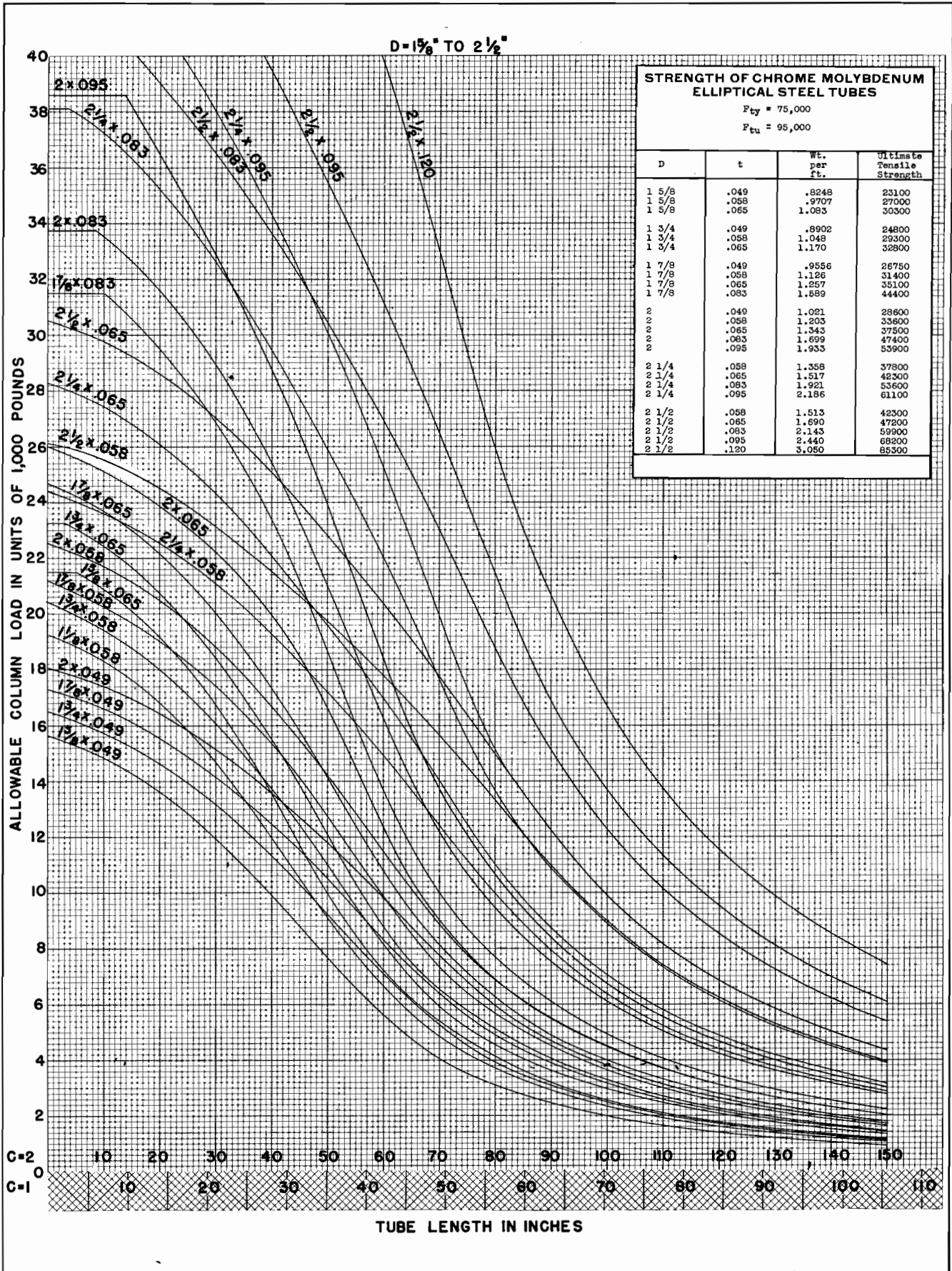
**STRENGTH OF CHROME MOLYBDENUM ELLIPTICAL STEEL TUBES**

$F_{ty} = 75,000$   
 $F_{tu} = 95,000$

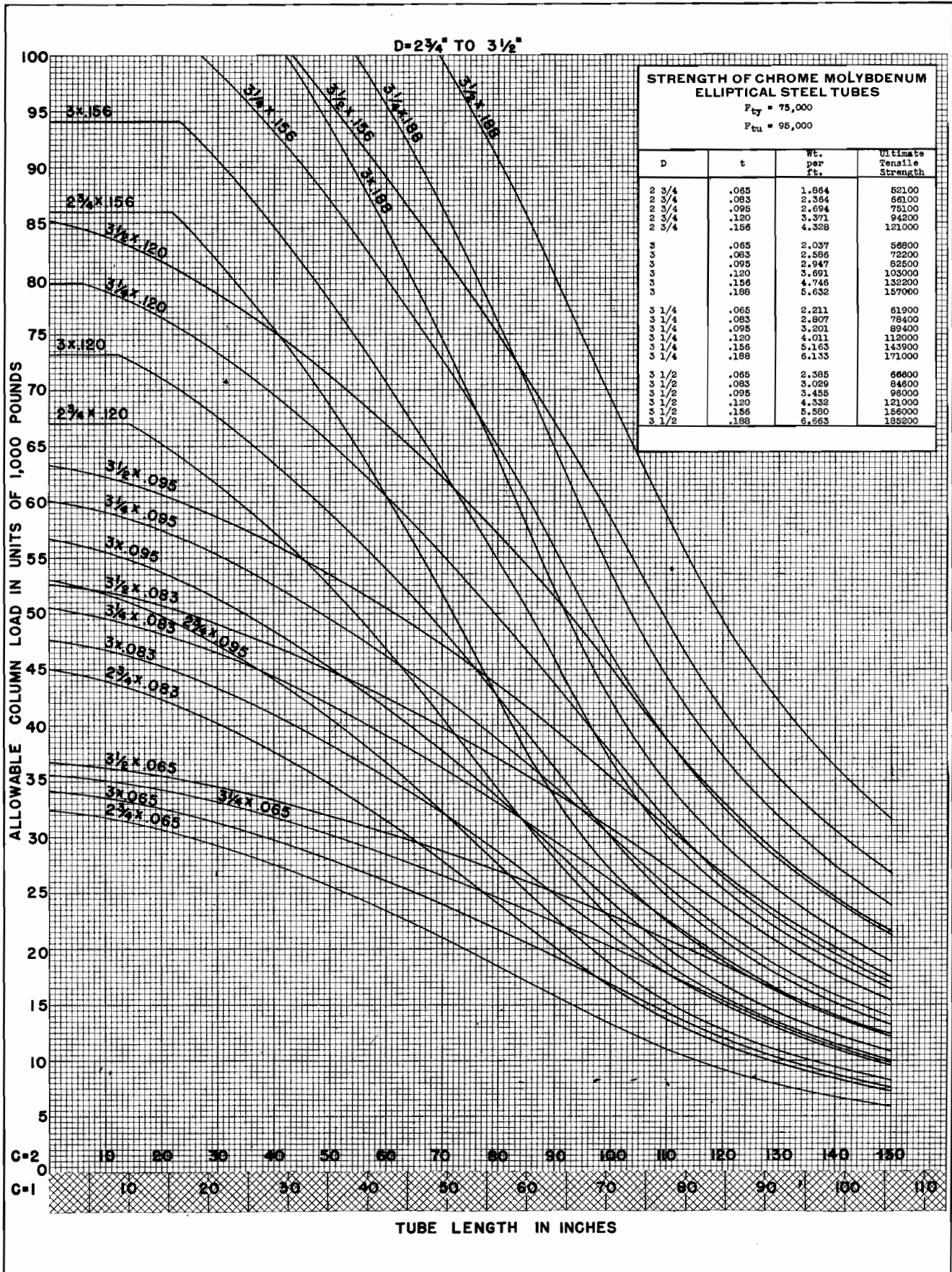
D	t	Wt. per ft.	Ultimate Tensile Strength
3/4	.035	.2973	7460
7/8	.035	.3140	8760
7/8	.049	.4323	12100
1	.025	.3607	10100
1	.049	.4977	13900
1 1/8	.035	.4074	11300
1 1/8	.049	.5631	15700
1 1/4	.035	.4542	12660
1 1/4	.049	.6285	17510
1 1/4	.058	.7384	20600
1 3/8	.035	.5009	14000
1 3/8	.049	.6939	19400
1 3/8	.058	.8158	22800
1 1/2	.035	.5476	15300
1 1/2	.049	.7593	21200
1 1/2	.058	.8932	25000
1 1/2	.065	.9962	27800

For Ovals — 85,000 Yield See III — 56



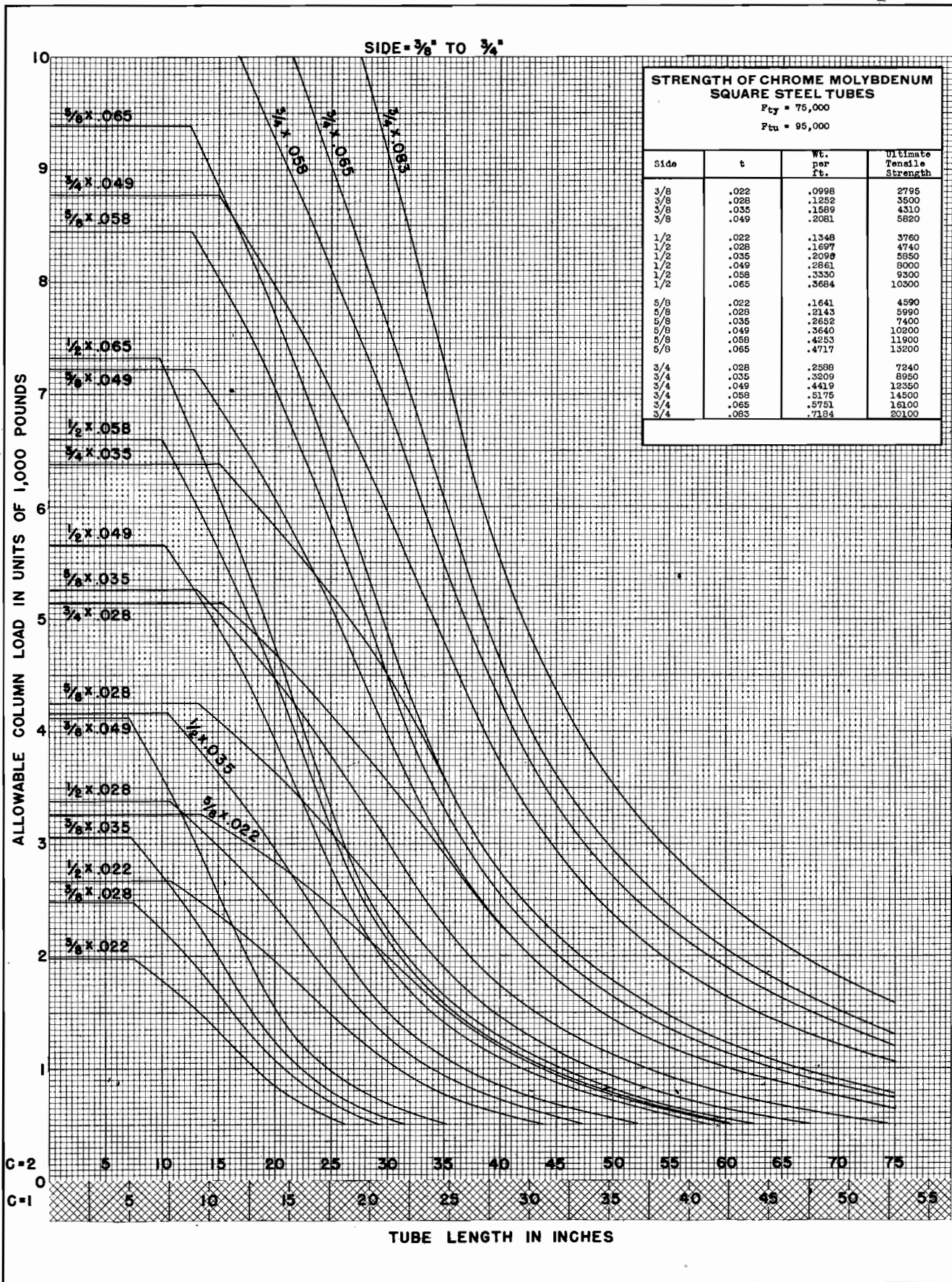


For Ovals — 85,000 Yield — See III — 56

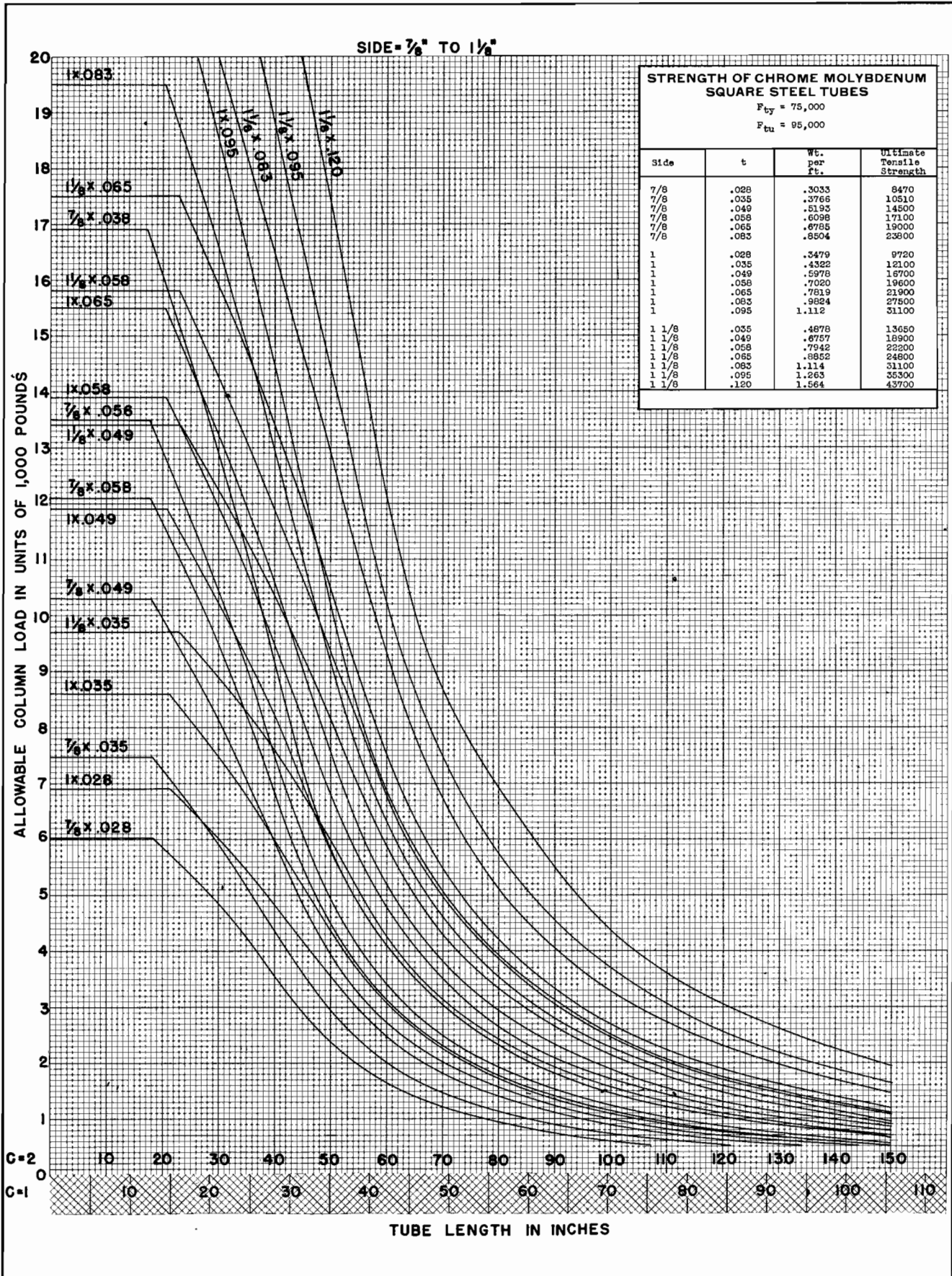


For Ovals — 85,000 Yield See III — 56



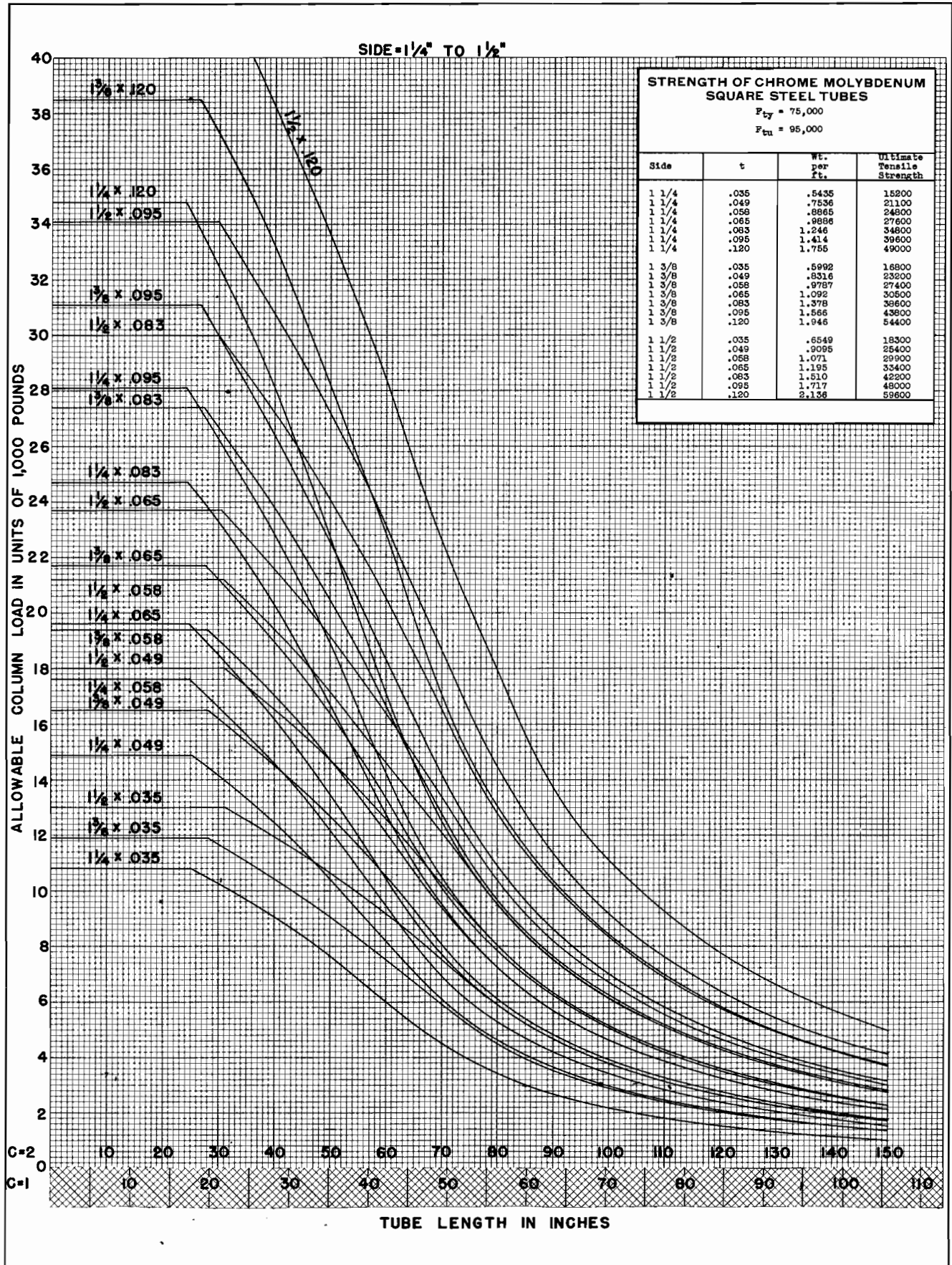


For Squares — 85,000 Yield See III — 59



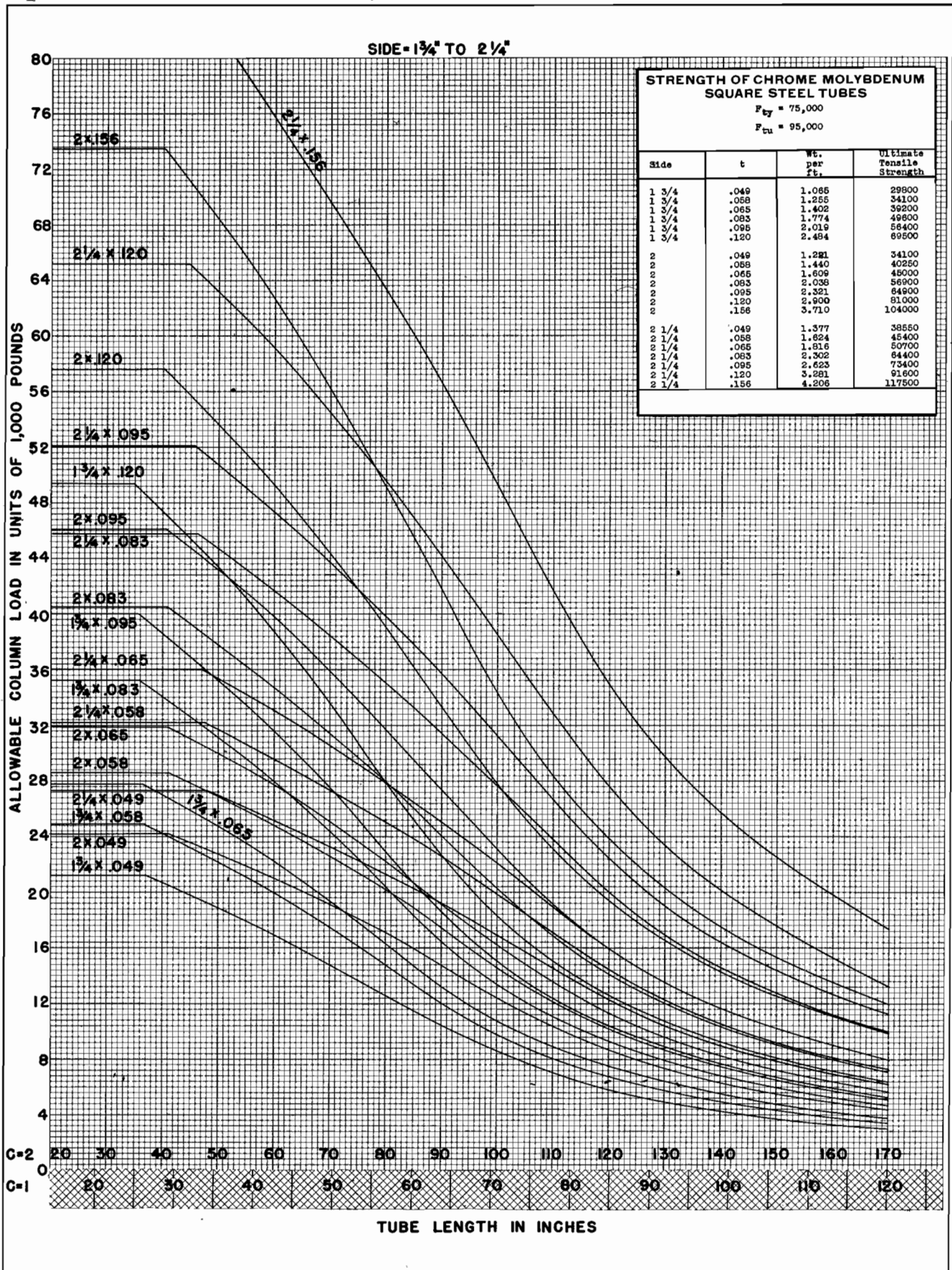
For Squares — 85,000 Yield See III — 59





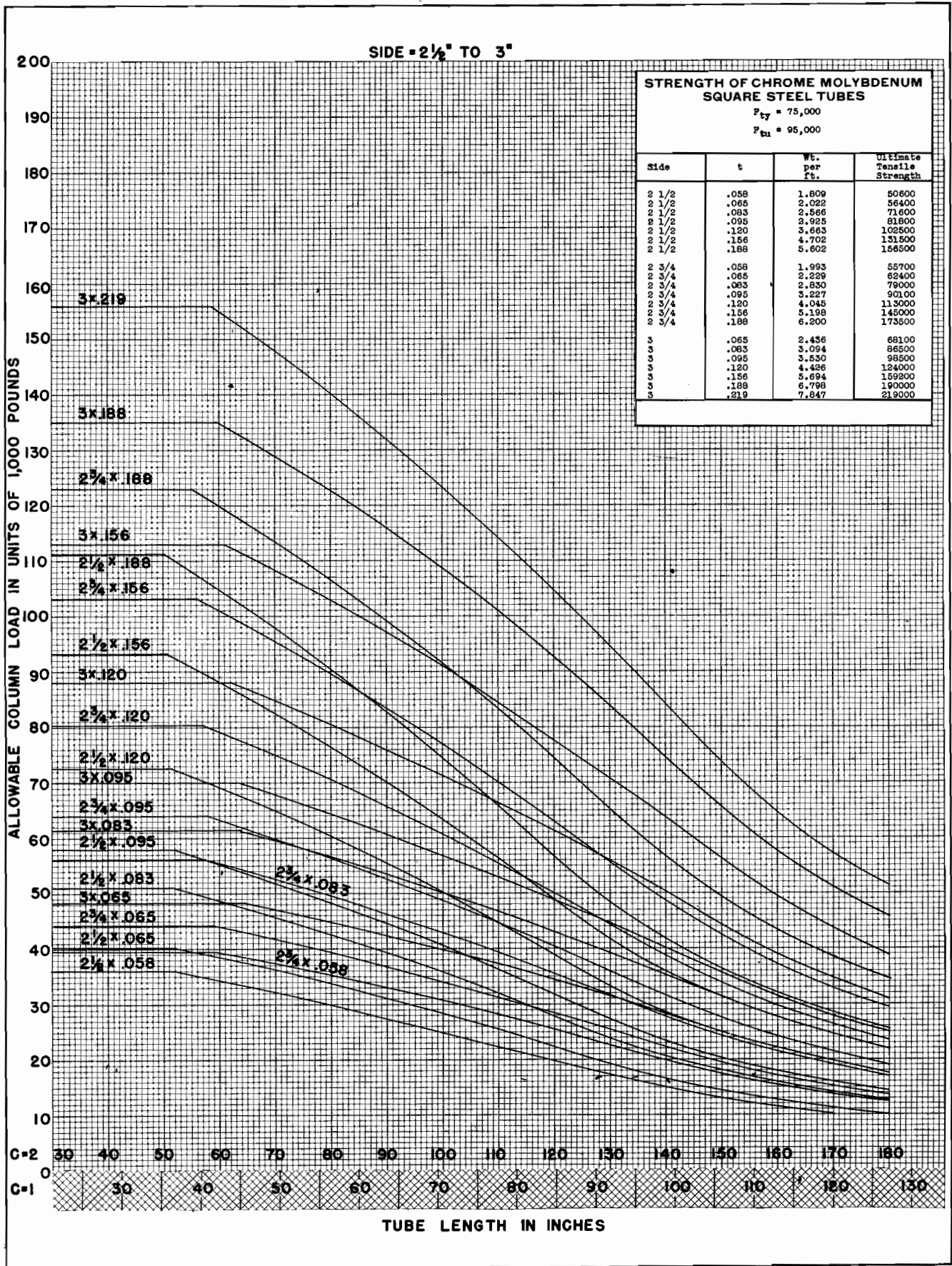
For Squares — 85,000 Yield See III — 59



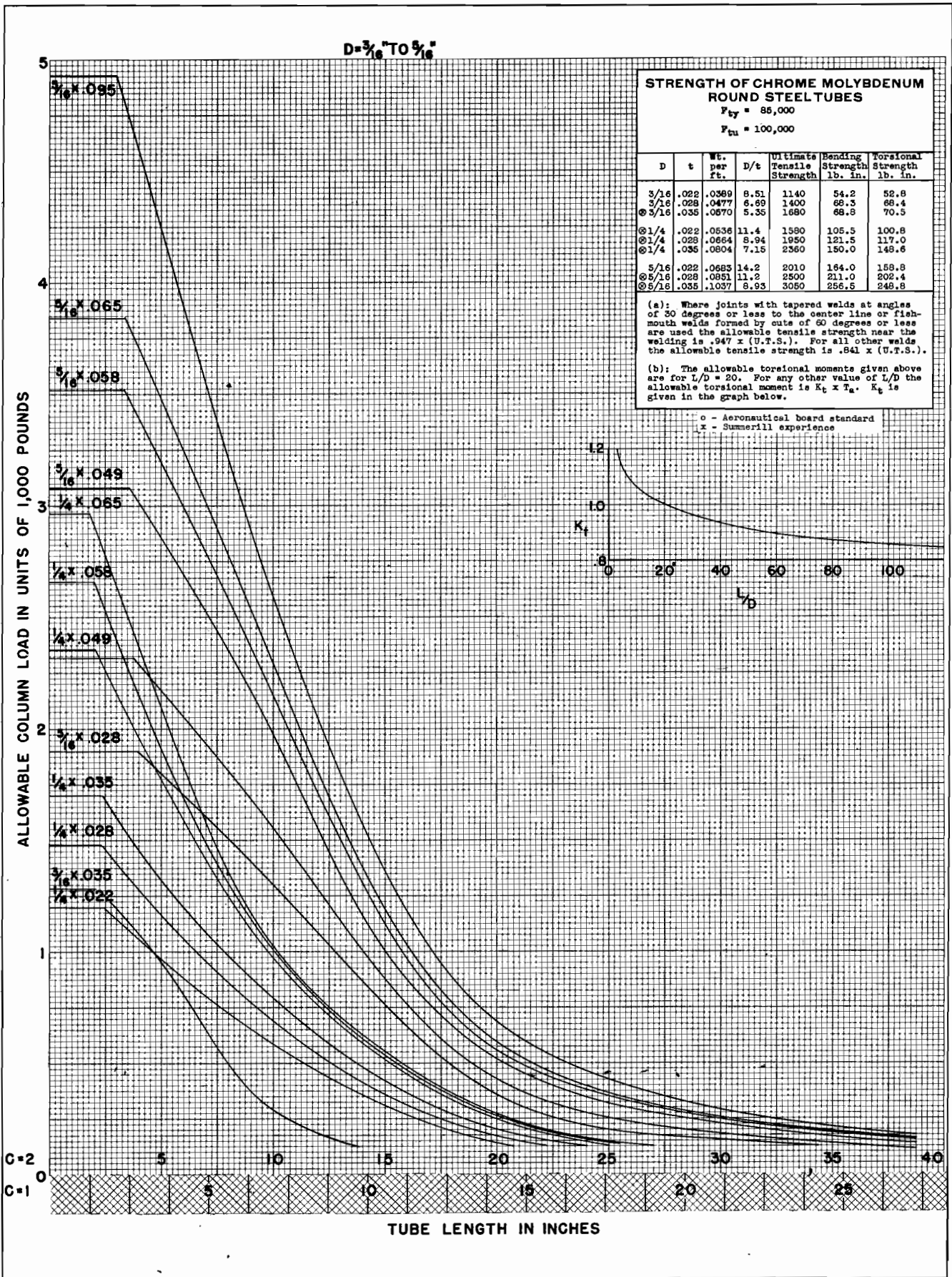


For Squares — 85,000 Yield See III — 59



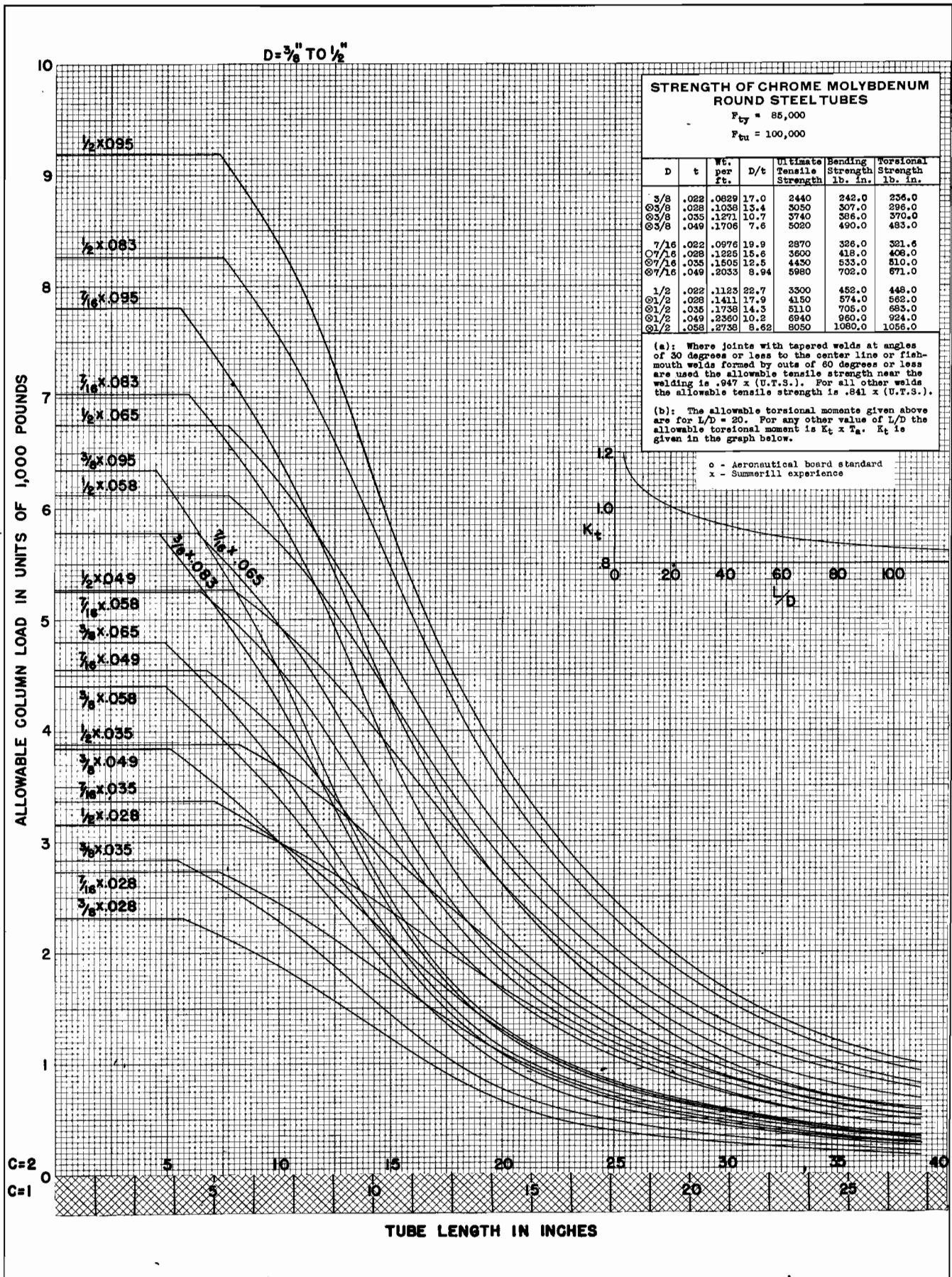


For Squares — 85,000 Yield See III — 59



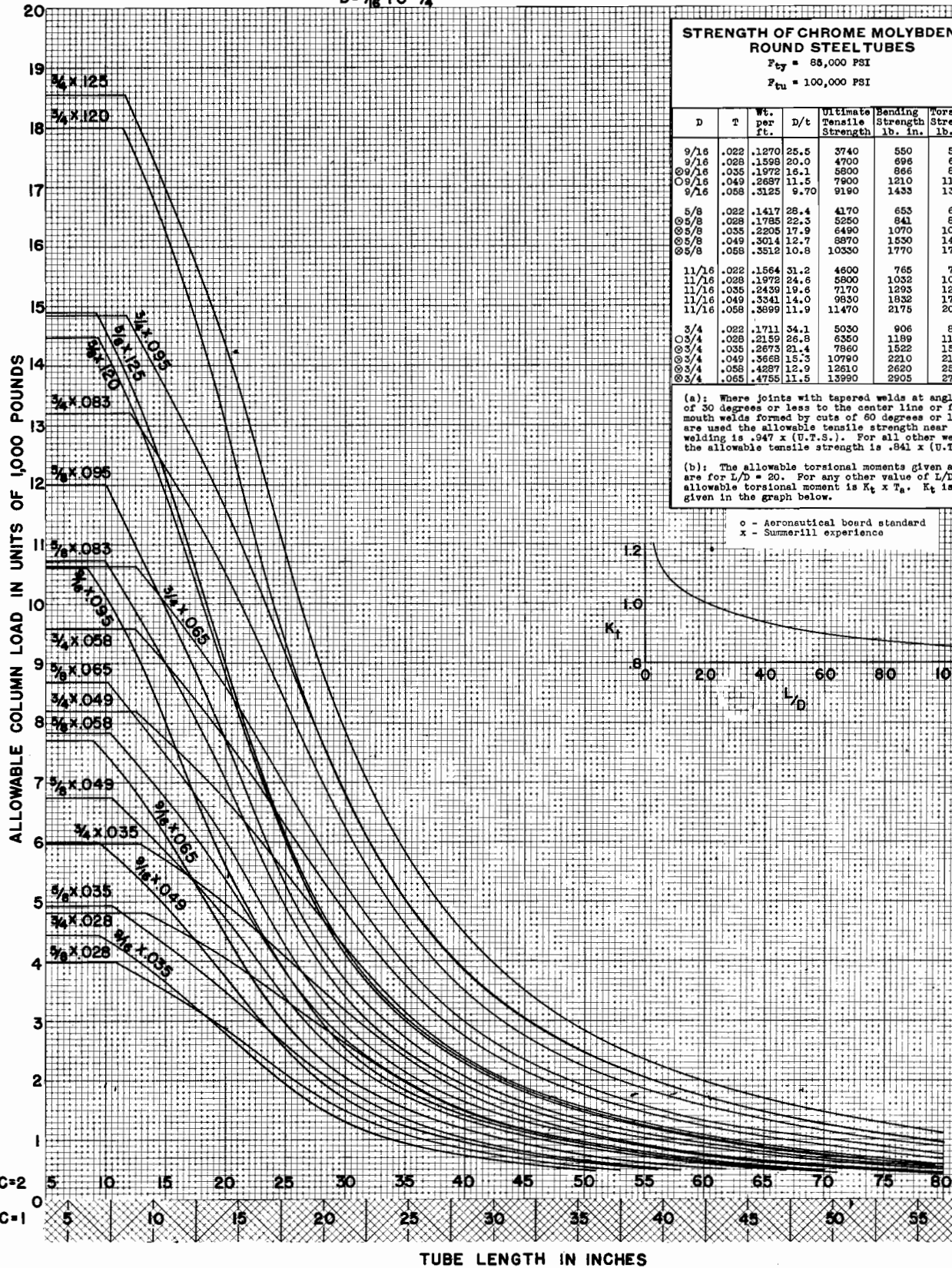
For Rounds — 75,000 Yield - See III — 20





For Rounds — 75,000 Yield — See III — 20

$D = \frac{9}{16}$  TO  $\frac{3}{4}$



**STRENGTH OF CHROME MOLYBDENUM ROUND STEEL TUBES**

$F_{ty} = 85,000$  PSI  
 $F_{tu} = 100,000$  PSI

D	T	Wt. per ft.	D/t	Ultimate Tensile Strength	Bending Strength lb. in.	Torsional Strength lb. in.
9/16	.022	1.270	25.5	3740	550	543
9/16	.028	1.598	20.0	4700	696	688
⊙9/16	.035	1.972	16.1	5800	866	848
⊙9/16	.049	2.687	11.5	7900	1210	1160
9/16	.058	3.125	9.70	9190	1433	1384
5/8	.022	1.417	28.4	4170	653	647
⊙5/8	.028	1.785	22.3	5250	841	830
⊙5/8	.035	2.205	17.9	6490	1070	1044
⊙5/8	.049	3.014	12.7	8870	1530	1462
⊙5/8	.058	3.512	10.8	10330	1770	1708
11/16	.022	1.564	31.2	4600	765	738
11/16	.028	1.972	24.6	5800	1032	1018
11/16	.035	2.439	19.6	7170	1293	1278
11/16	.049	3.341	14.0	9630	1832	1770
11/16	.058	3.999	11.9	11470	2175	2088
3/4	.022	1.711	34.1	5030	906	880
⊙3/4	.028	2.159	26.8	6350	1189	1170
⊙3/4	.035	2.673	21.4	7860	1522	1502
⊙3/4	.049	3.668	15.3	10790	2210	2154
⊙3/4	.058	4.287	12.9	12610	2620	2518
⊙3/4	.065	4.755	11.5	13990	2905	2784

(a): Where joints with tapered welds at angles of 30 degrees or less to the center line or fish-mouth welds formed by cuts of 60 degrees or less are used the allowable tensile strength near the welding is .947 x (U.T.S.). For all other welds the allowable tensile strength is .841 x (U.T.S.).

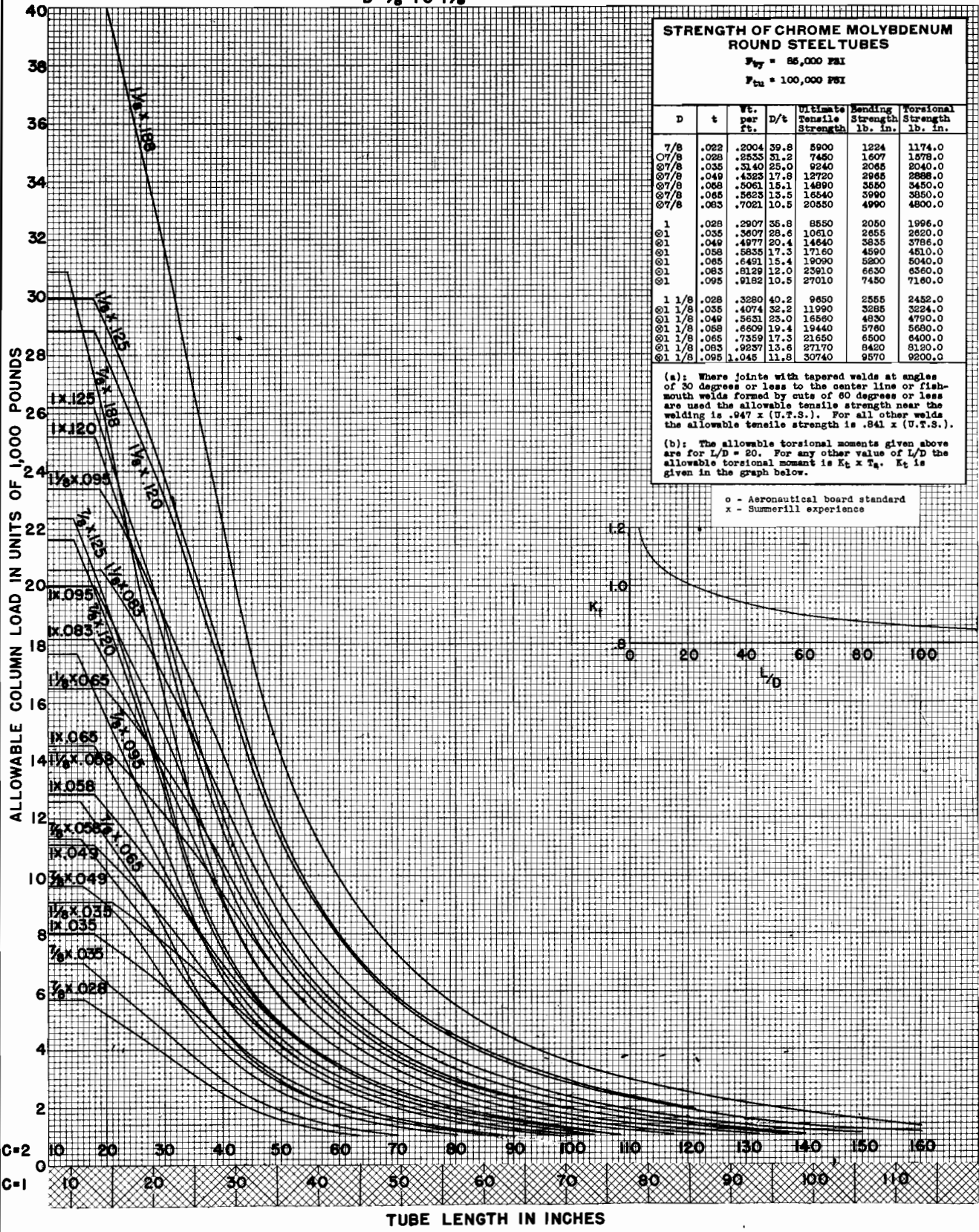
(b): The allowable torsional moments given above are for  $L/D = 20$ . For any other value of  $L/D$  the allowable torsional moment is  $K_t \times T_A$ .  $K_t$  is given in the graph below.

o - Aeronautical board standard  
 x - Summerill experience

For Rounds — 75,000 Yield . See III — 20



D = 7/8" TO 1 1/8"



For Rounds - 75,000 Yield - See III - 20

D = 1/4" TO 1 1/2"

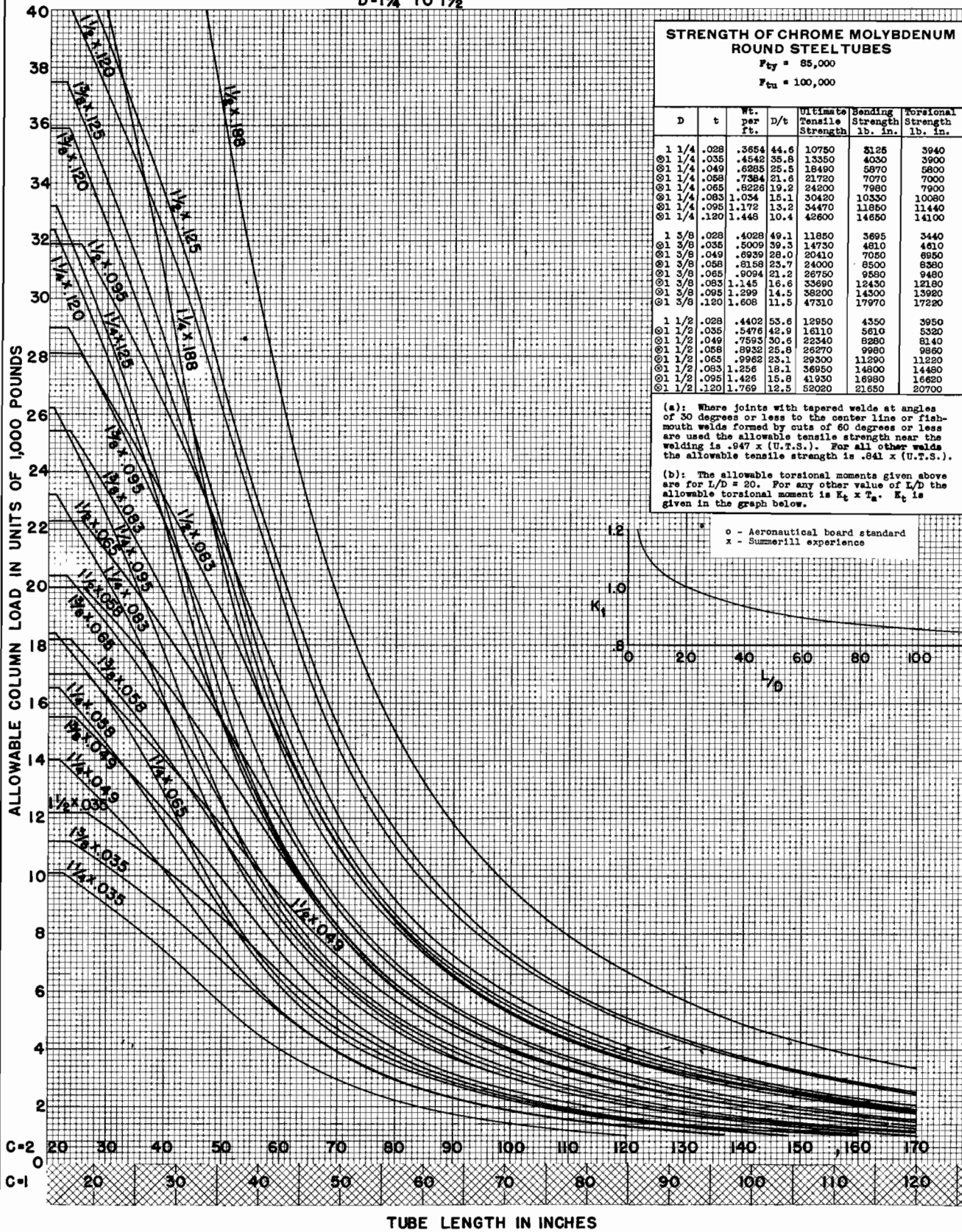
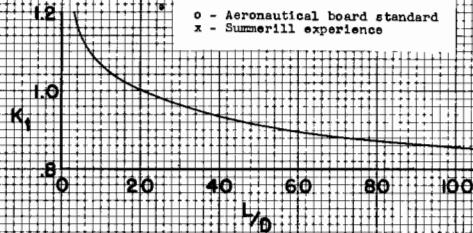
**STRENGTH OF CHROME MOLYBDENUM ROUND STEEL TUBES**

$F_{ty} = 85,000$   
 $F_{tu} = 100,000$

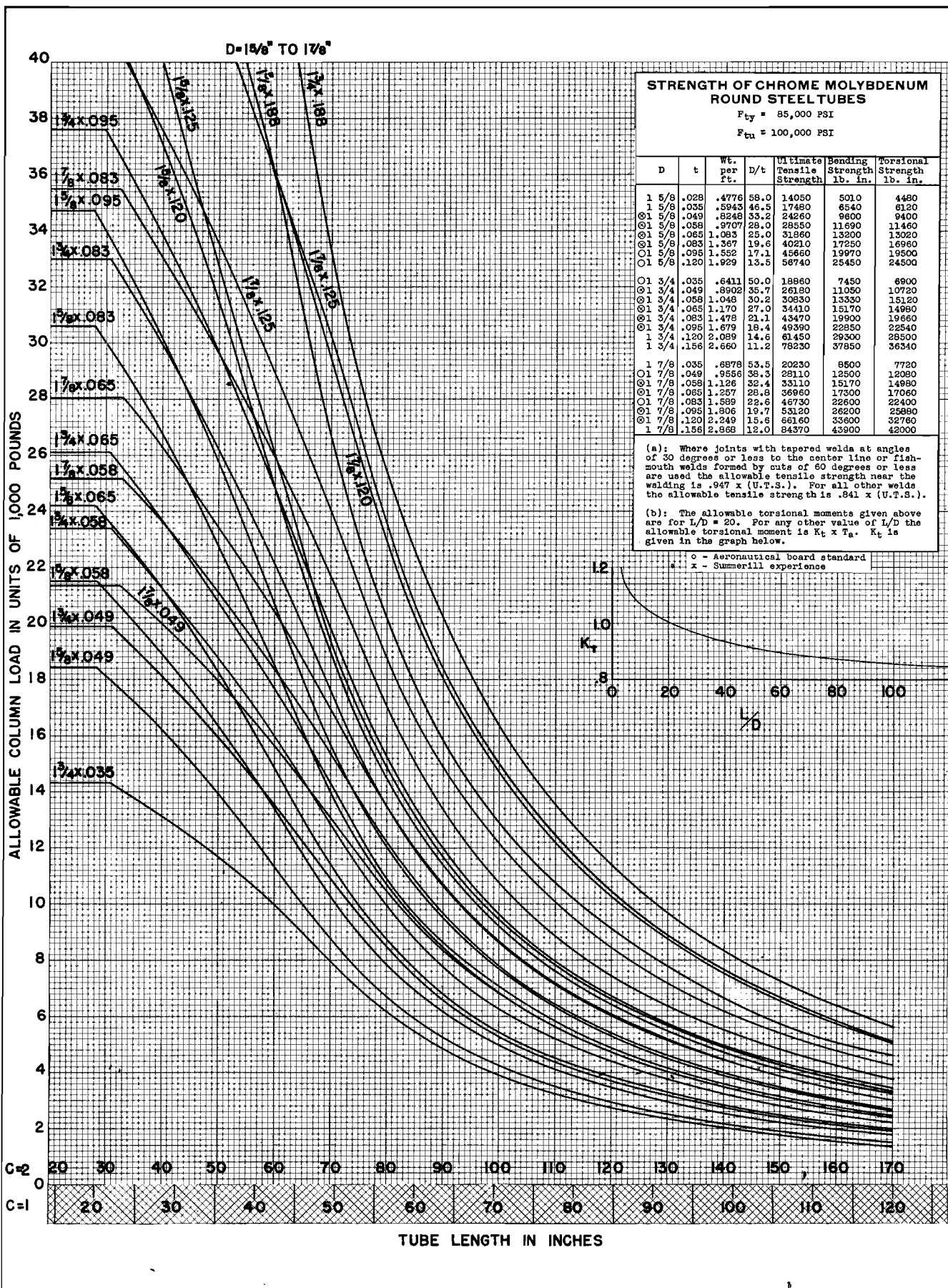
D	t	Wt. per ft.	D/t	Ultimate Tensile Strength	Bending Strength lb. in.	Torsional Strength lb. in.
1 1/4	.028	.3654	44.6	10750	5126	3940
⊗ 1 1/4	.035	.4542	35.8	13350	4030	3900
⊗ 1 1/4	.049	.6285	25.5	18490	5370	5800
⊗ 1 1/4	.058	.7384	21.6	21720	7070	7000
⊗ 1 1/4	.065	.8226	19.2	24200	7980	7900
⊗ 1 1/4	.083	1.034	15.1	30420	10330	10080
⊗ 1 1/4	.096	1.172	13.2	34470	11850	11440
⊗ 1 1/4	.120	1.448	10.4	42600	14650	14100
1 3/8	.028	.4028	49.1	11850	3695	3440
⊗ 1 3/8	.035	.5009	39.3	14730	4810	4610
⊗ 1 3/8	.049	.6929	28.0	20410	7050	6950
⊗ 1 3/8	.058	.8158	23.7	24000	8530	8320
⊗ 1 3/8	.065	.9094	21.2	26750	9580	9480
⊗ 1 3/8	.083	1.145	16.6	33690	12430	12180
⊗ 1 3/8	.095	1.299	14.5	38200	14300	13920
⊗ 1 3/8	.120	1.608	11.5	47310	17970	17220
1 1/2	.028	.4402	53.6	12950	4350	3950
⊗ 1 1/2	.035	.5476	42.9	16110	5610	5320
⊗ 1 1/2	.049	.7593	30.6	22340	8280	8140
⊗ 1 1/2	.058	.8932	25.8	26270	9980	9860
⊗ 1 1/2	.065	.9962	23.1	29300	11290	11220
⊗ 1 1/2	.083	1.256	18.1	36850	14800	14450
⊗ 1 1/2	.095	1.426	15.8	41930	16980	16620
⊗ 1 1/2	.120	1.769	12.5	52020	21650	20700

(a) Where joints with tapered welds at angles of 30 degrees or less to the center line or fish-mouth welds formed by cuts of 60 degrees or less are used the allowable tensile strength near the welding is .947 x (U.T.S.). For all other welds the allowable tensile strength is .841 x (U.T.S.).

(b) The allowable torsional moments given above are for L/D = 20. For any other value of L/D the allowable torsional moment is  $K_t \times T_a$ .  $K_t$  is given in the graph below.



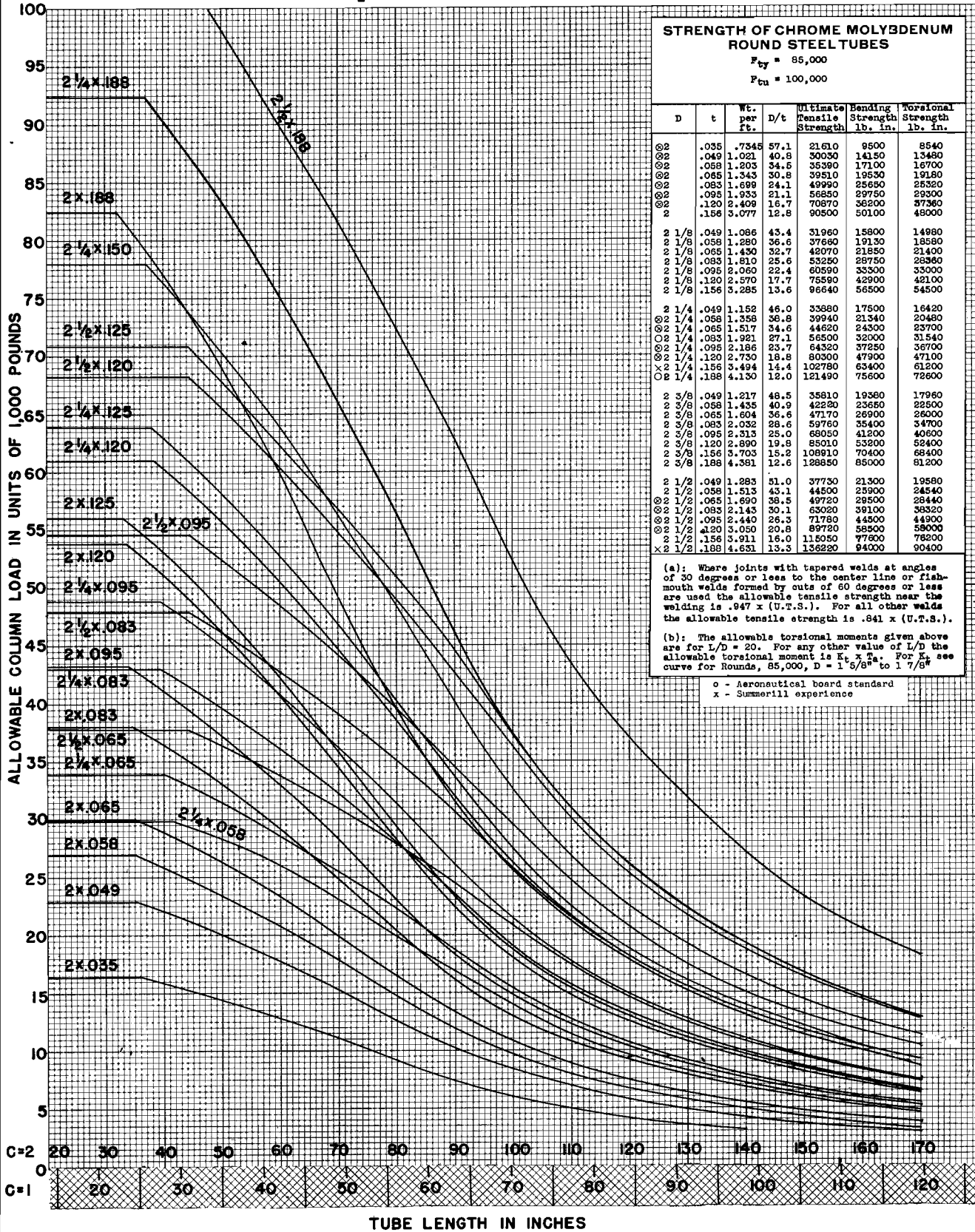
For Rounds — 75,000 Yield See III — 20



For Rounds — 75,000 Yield See III — 20

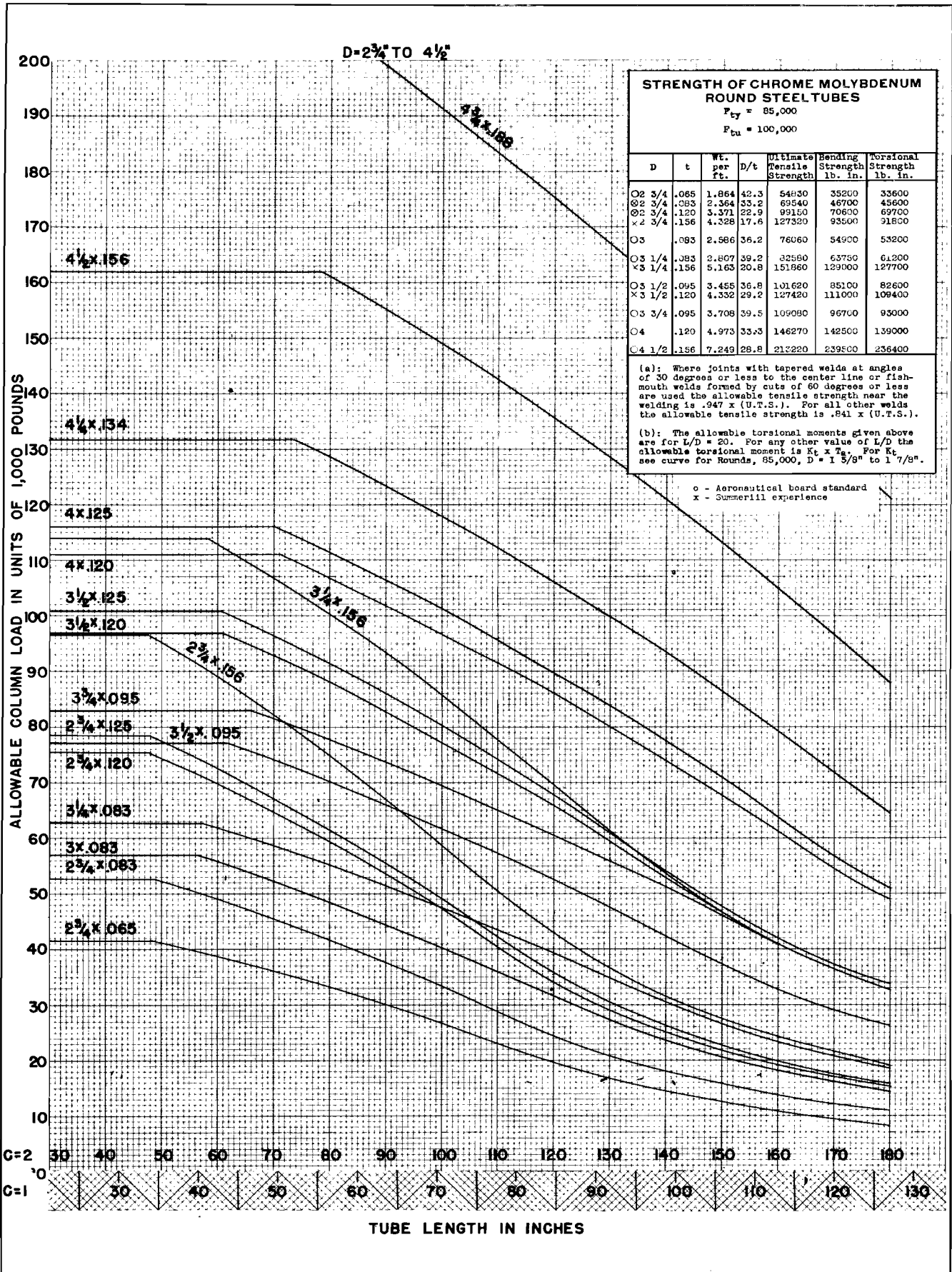


D=2" TO 2 1/2"



For Rounds — 75,000 Yield See III — 20

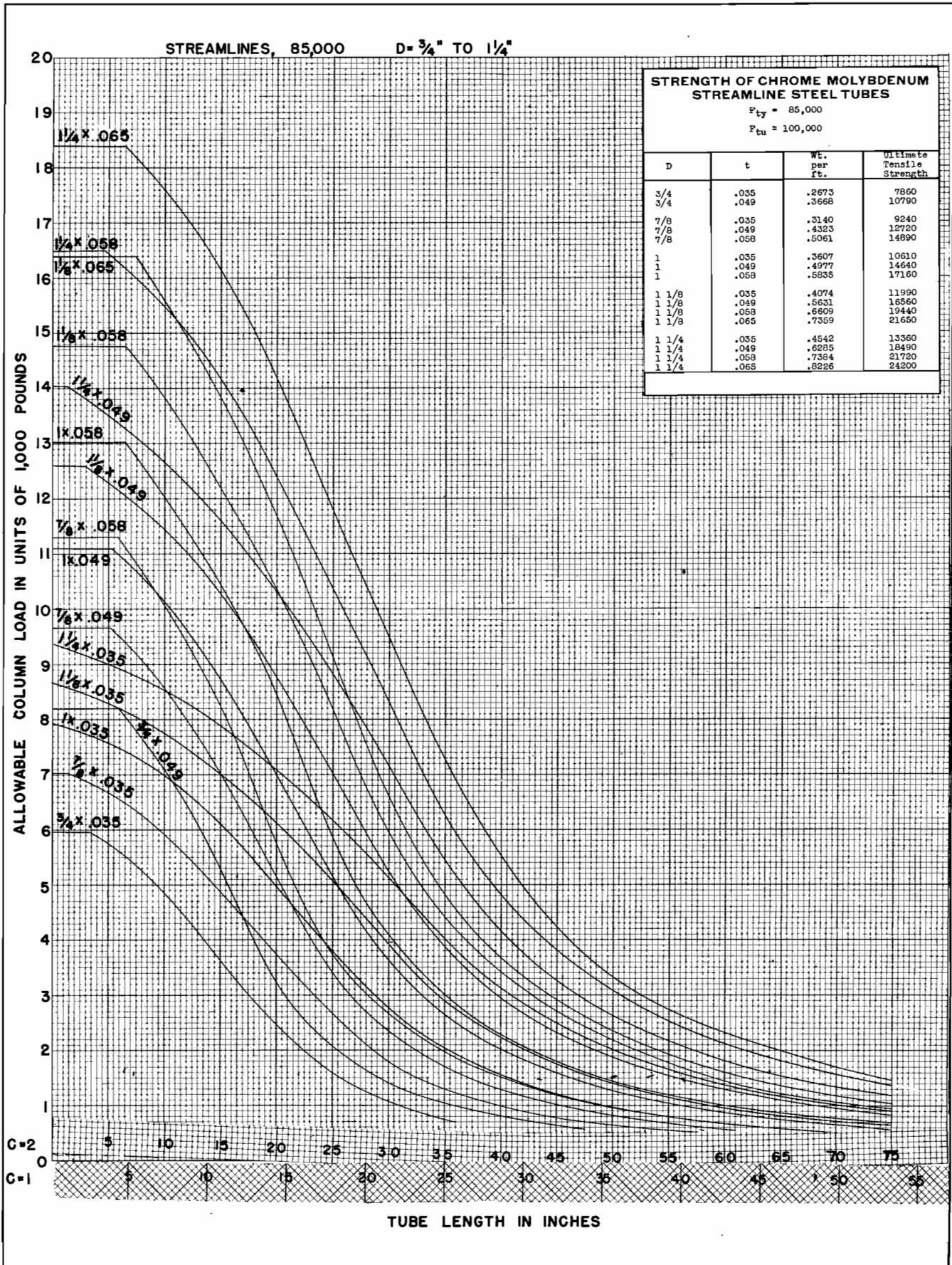




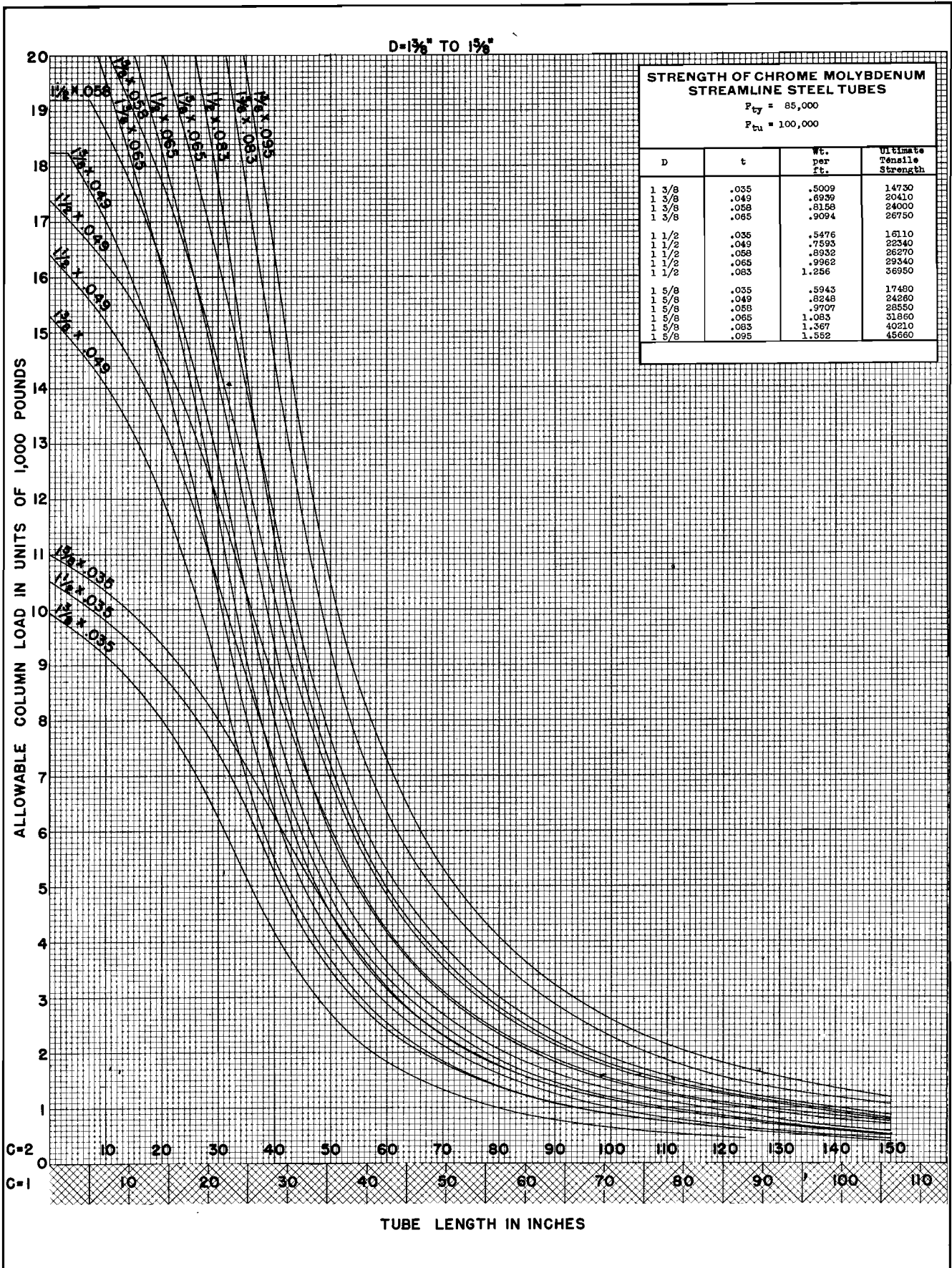
For Rounds — 75,000 Yield See III — 20

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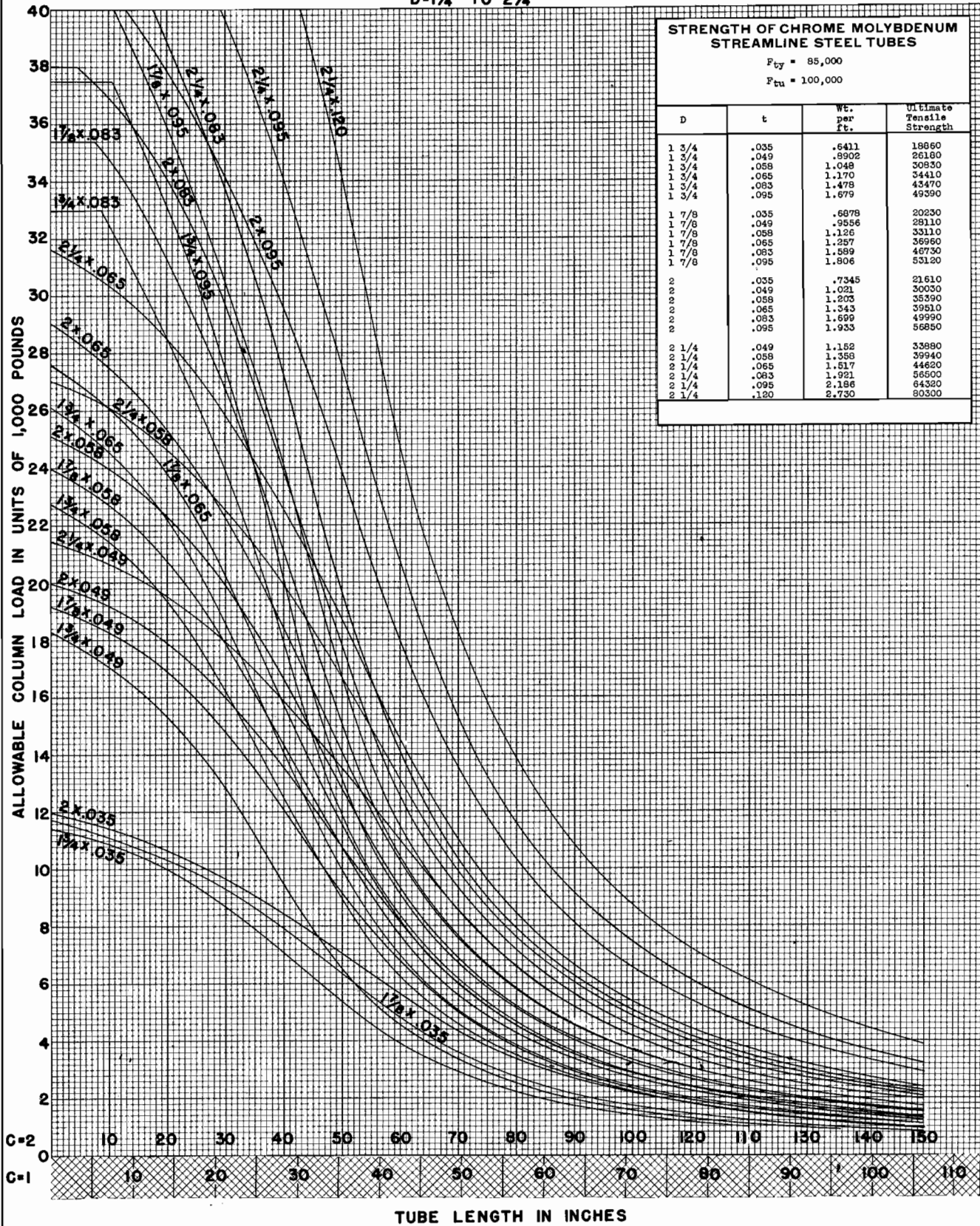


For Streamline — 75,000 Yield See III — 28



For Streamline — 75,000 Yield See III — 28

D=1 3/4" TO 2 1/4"



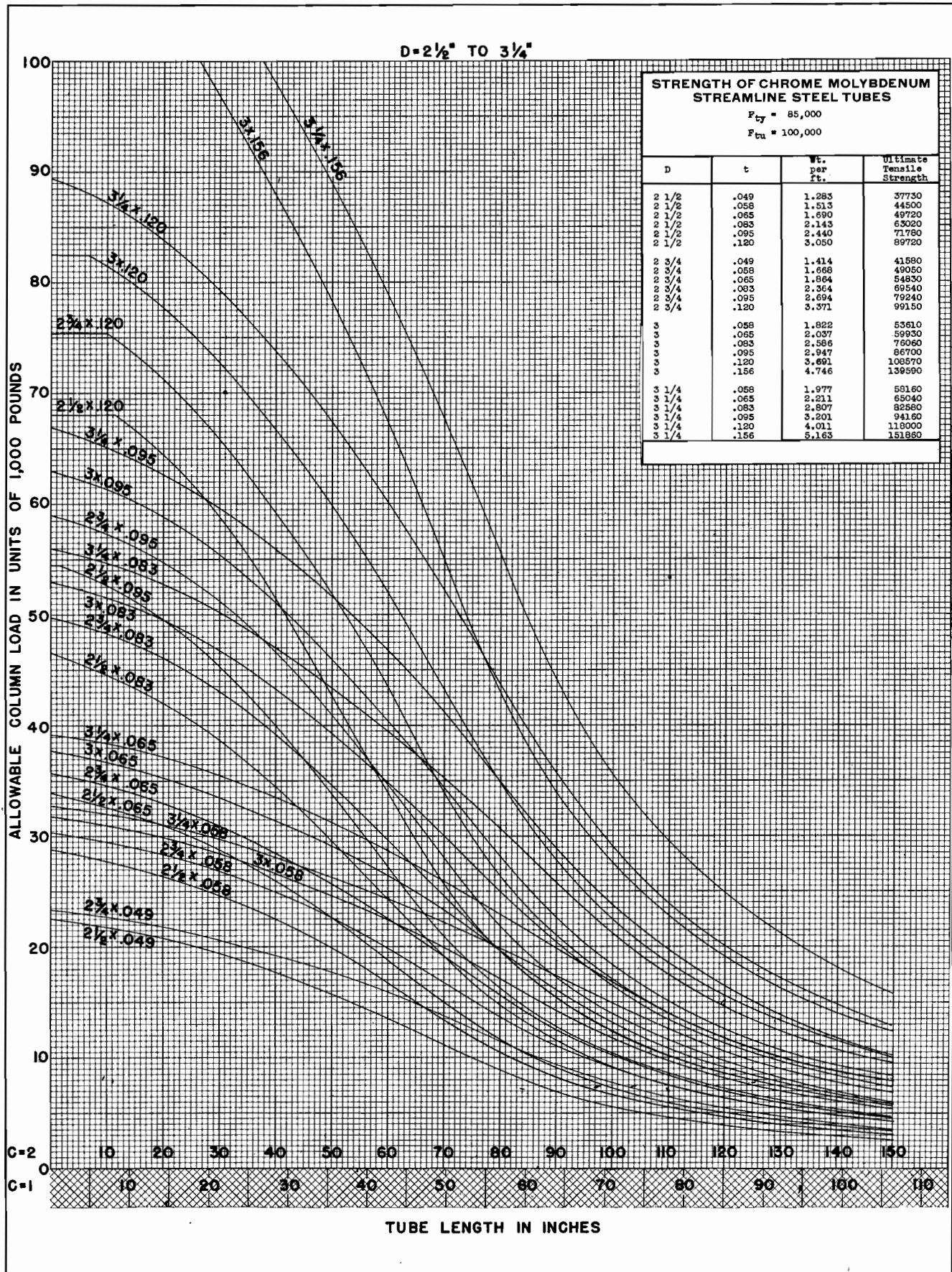
**STRENGTH OF CHROME MOLYBDENUM STREAMLINE STEEL TUBES**

F<sub>cy</sub> = 85,000  
F<sub>tu</sub> = 100,000

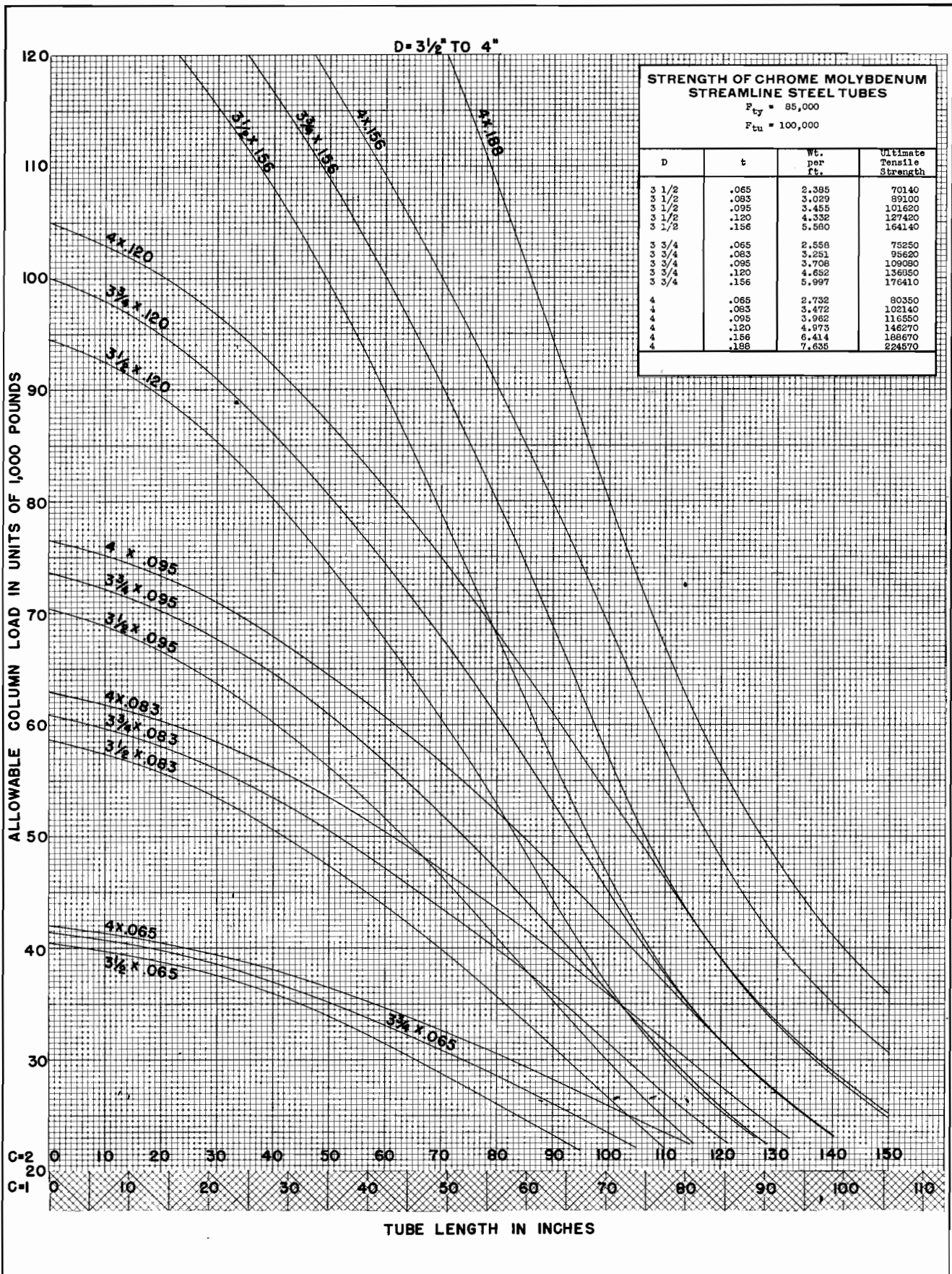
D	t	WE. per ft.	Ultimate Tensile Strength
1 3/4	.035	.6411	18860
1 3/4	.049	.8902	26180
1 3/4	.058	1.048	30830
1 3/4	.065	1.170	34410
1 3/4	.083	1.478	43470
1 3/4	.095	1.679	49390
1 7/8	.035	.6878	20230
1 7/8	.049	.9556	28110
1 7/8	.058	1.126	33110
1 7/8	.065	1.257	36960
1 7/8	.083	1.589	46730
1 7/8	.095	1.806	53120
2	.035	.7345	21510
2	.049	1.021	30030
2	.058	1.203	35390
2	.065	1.343	39510
2	.083	1.699	49990
2	.095	1.933	56850
2 1/4	.049	1.152	33880
2 1/4	.058	1.358	39940
2 1/4	.065	1.517	44620
2 1/4	.083	1.921	56500
2 1/4	.095	2.186	64320
2 1/4	.120	2.730	80300

For Streamline — 75,000 Yield See III — 28



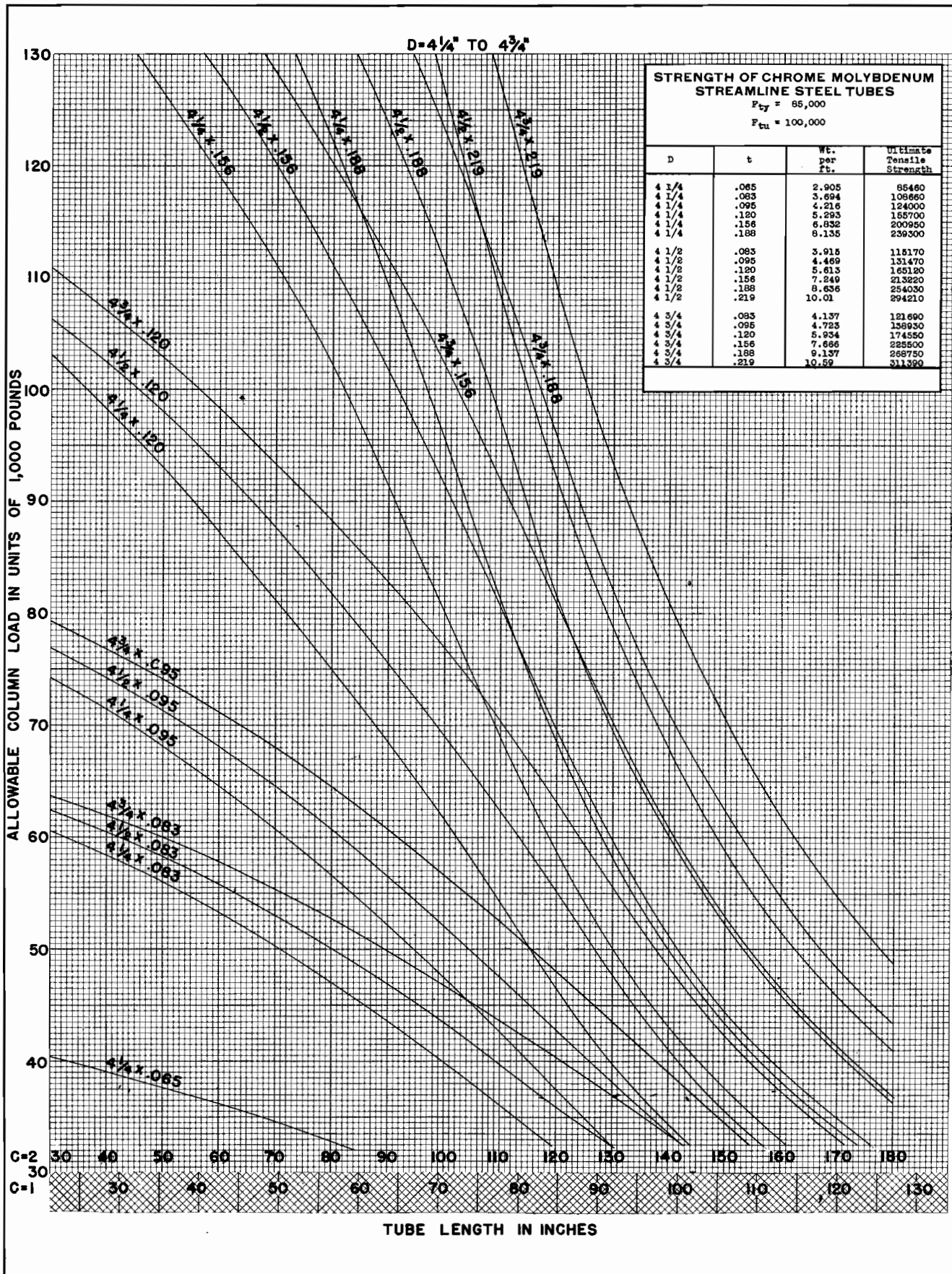


For Streamline — 75,000 Yield See III — 28



For Streamline — 75,000 Yield See III — 28

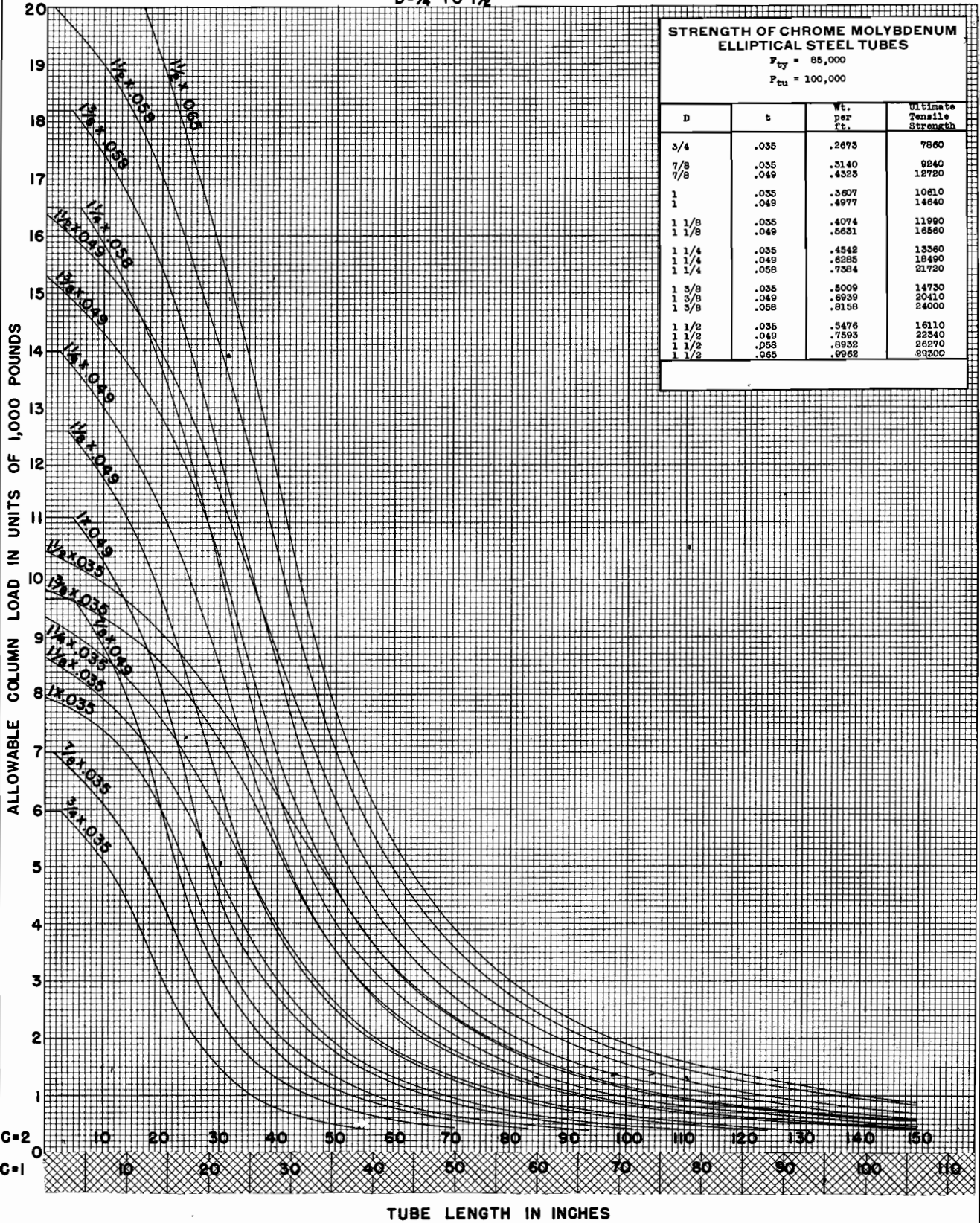




For Streamline — 75,000 Yield See III — 28



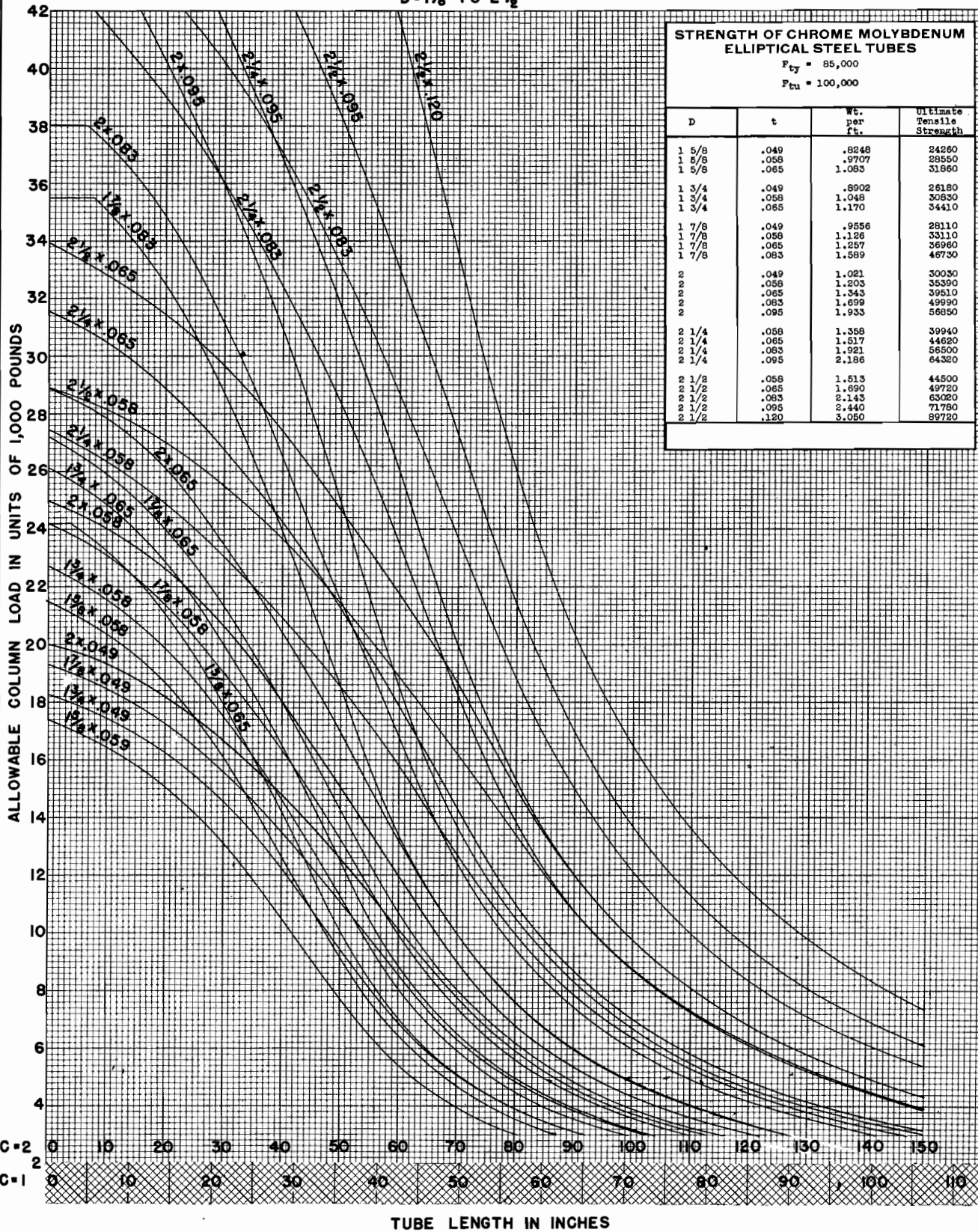
$D = \frac{3}{4}''$  TO  $1\frac{1}{2}''$



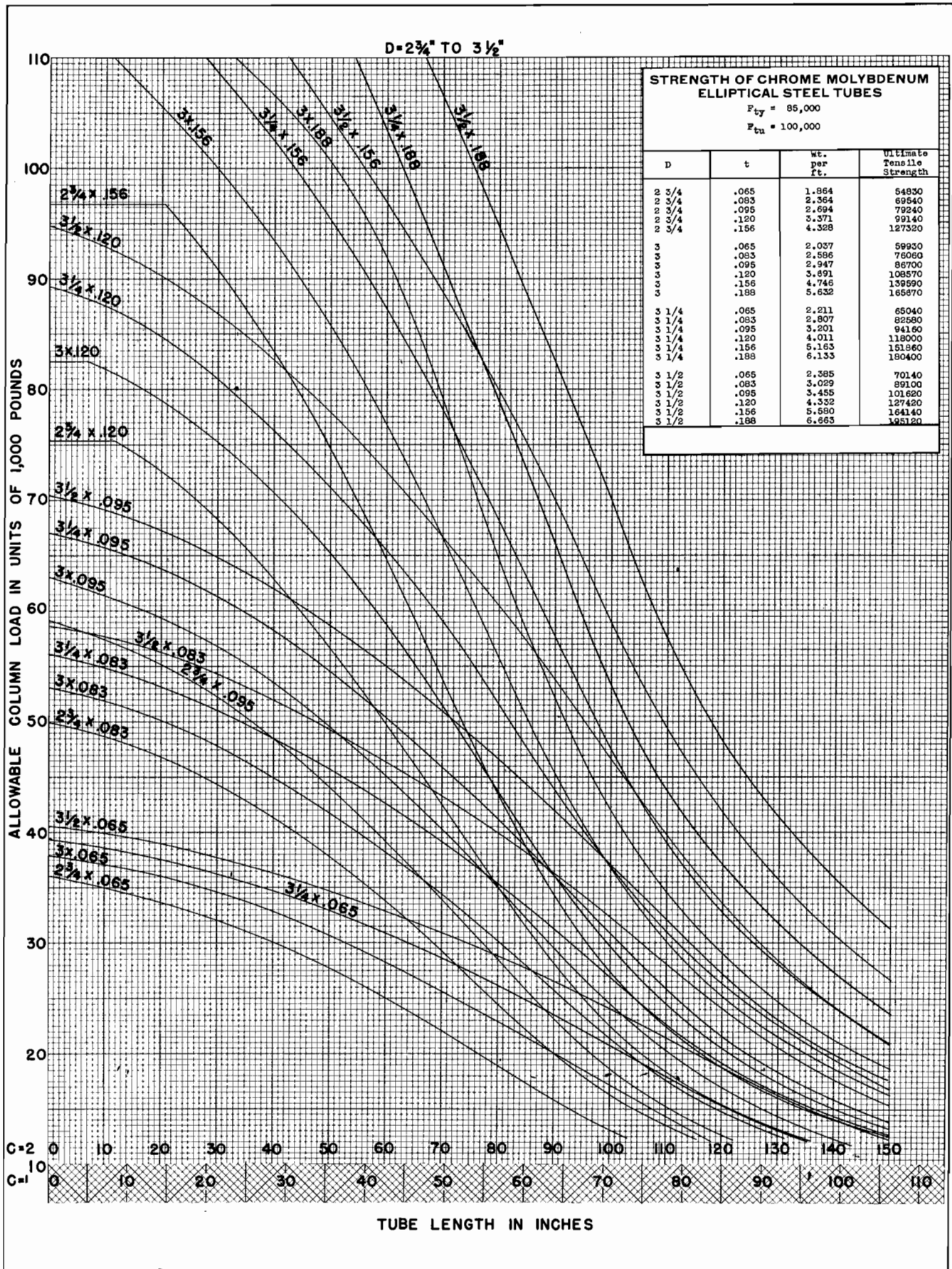
For Ovals — 75,000 Yield See III — 34



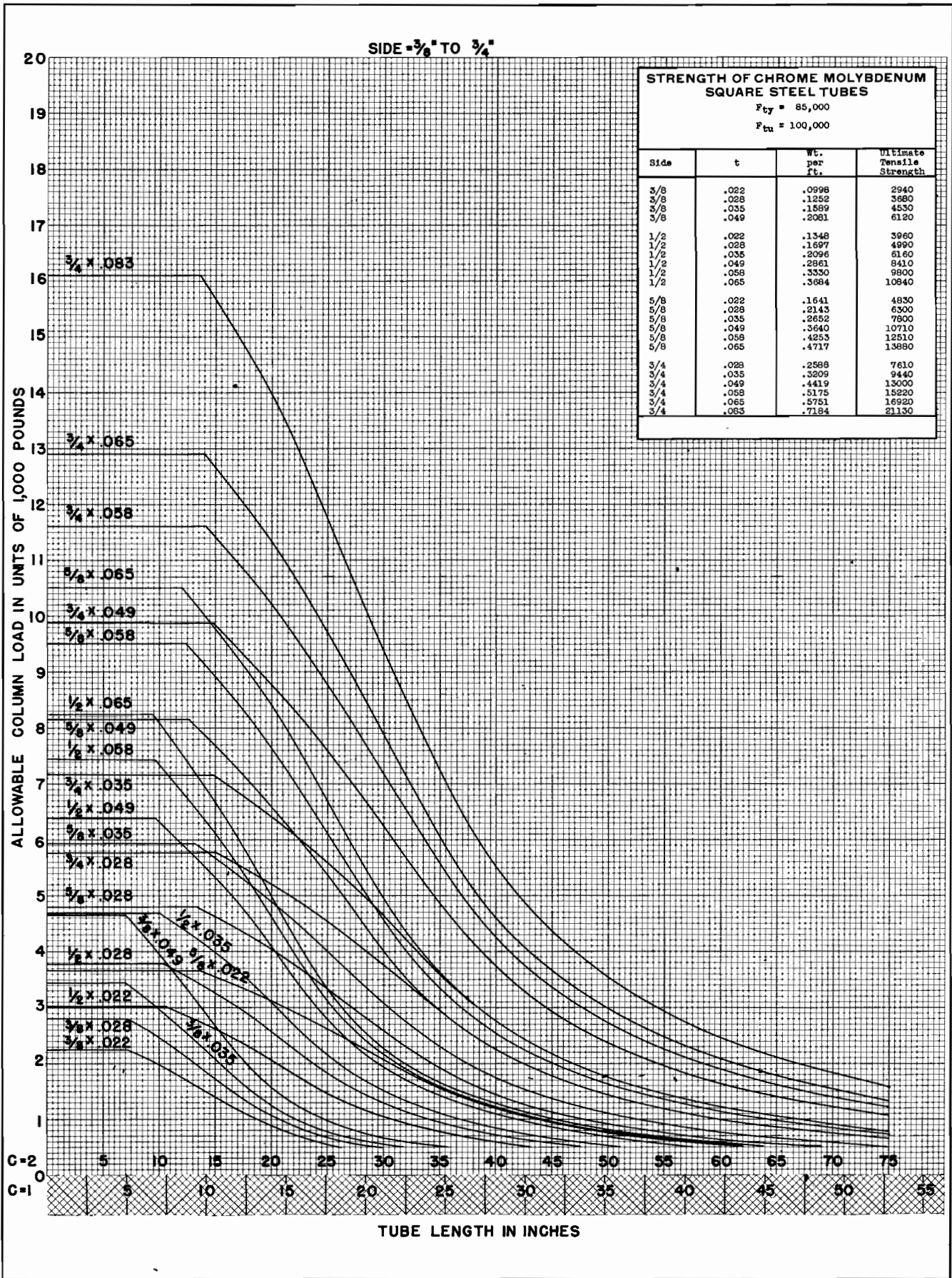
D = 1 5/8" TO 2 1/2"



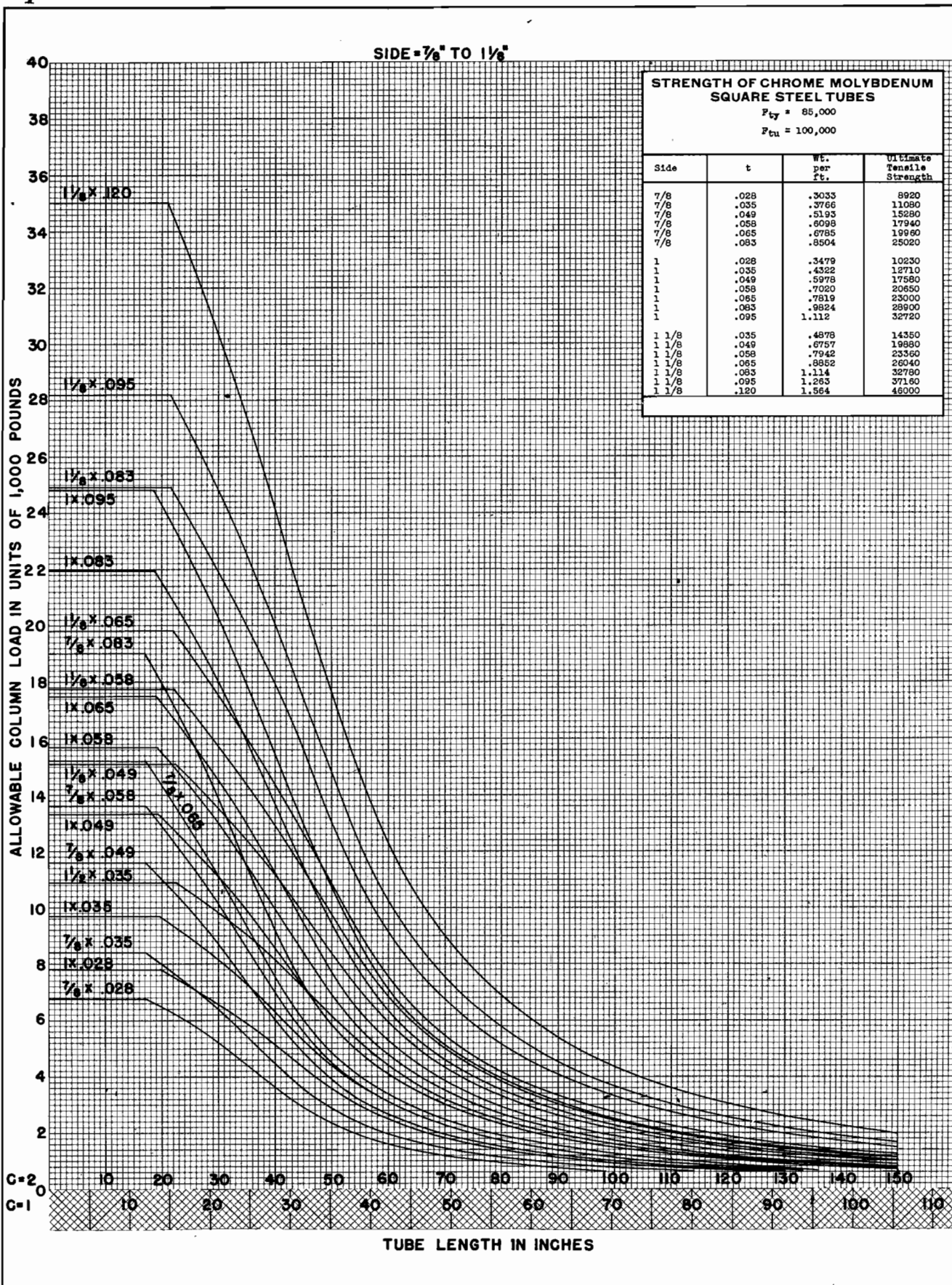
For Ovals — 75,000 Yield See III — 34



For Ovals — 75,000 Yield See III — 34

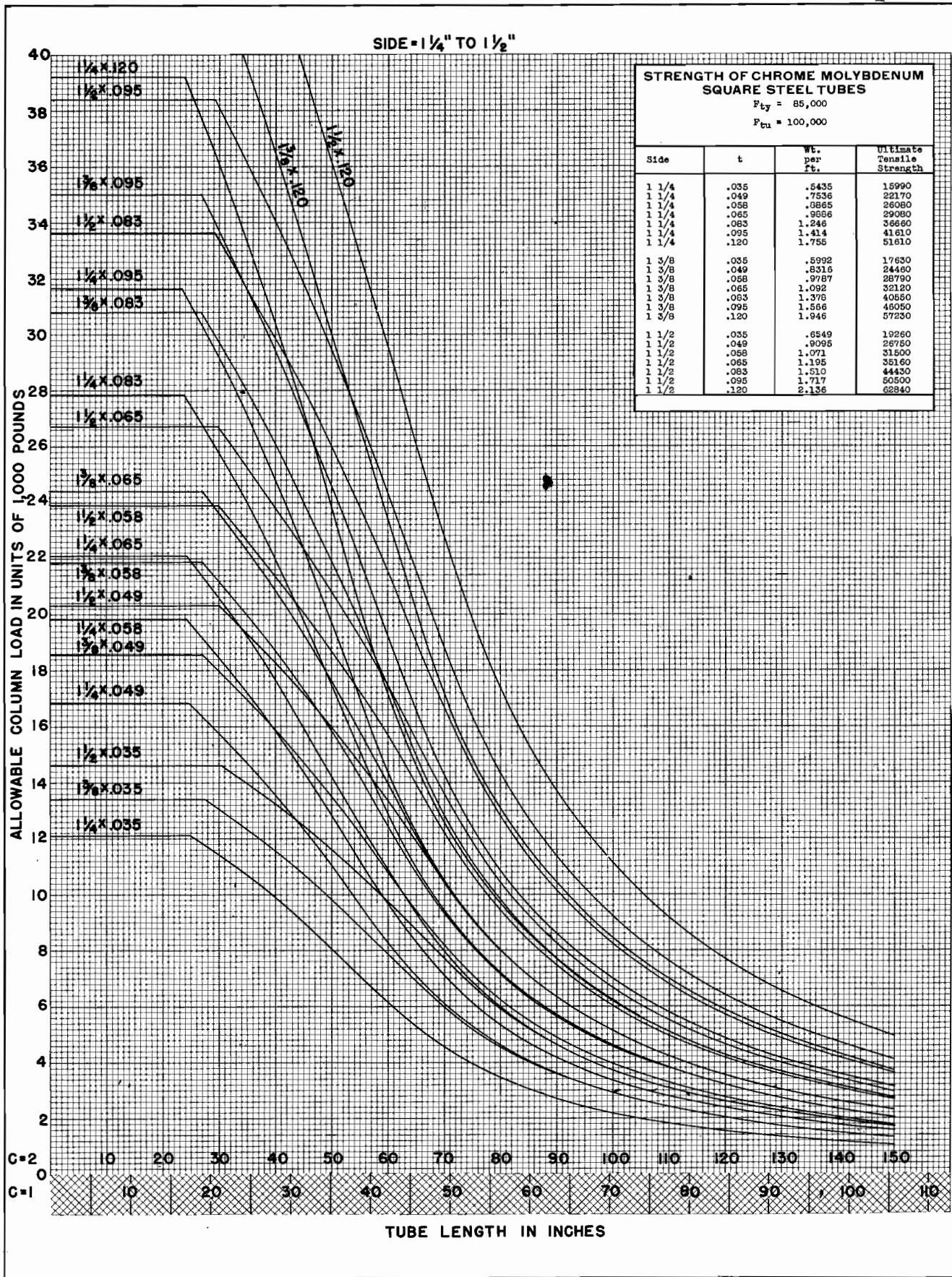


For Squares — 75,000 Yield - See III — 37

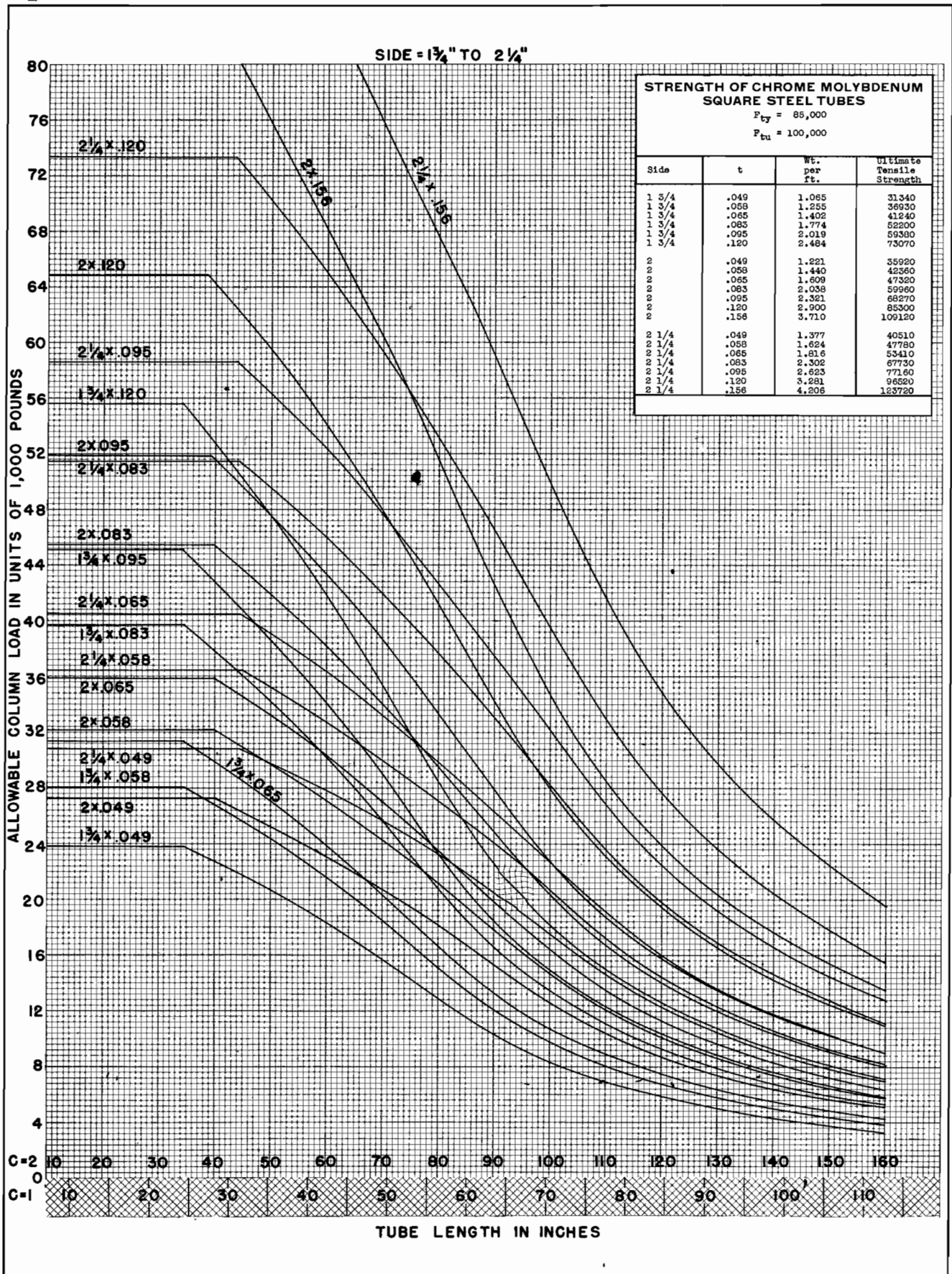


For Squares — 75,000 Yield See III — 37

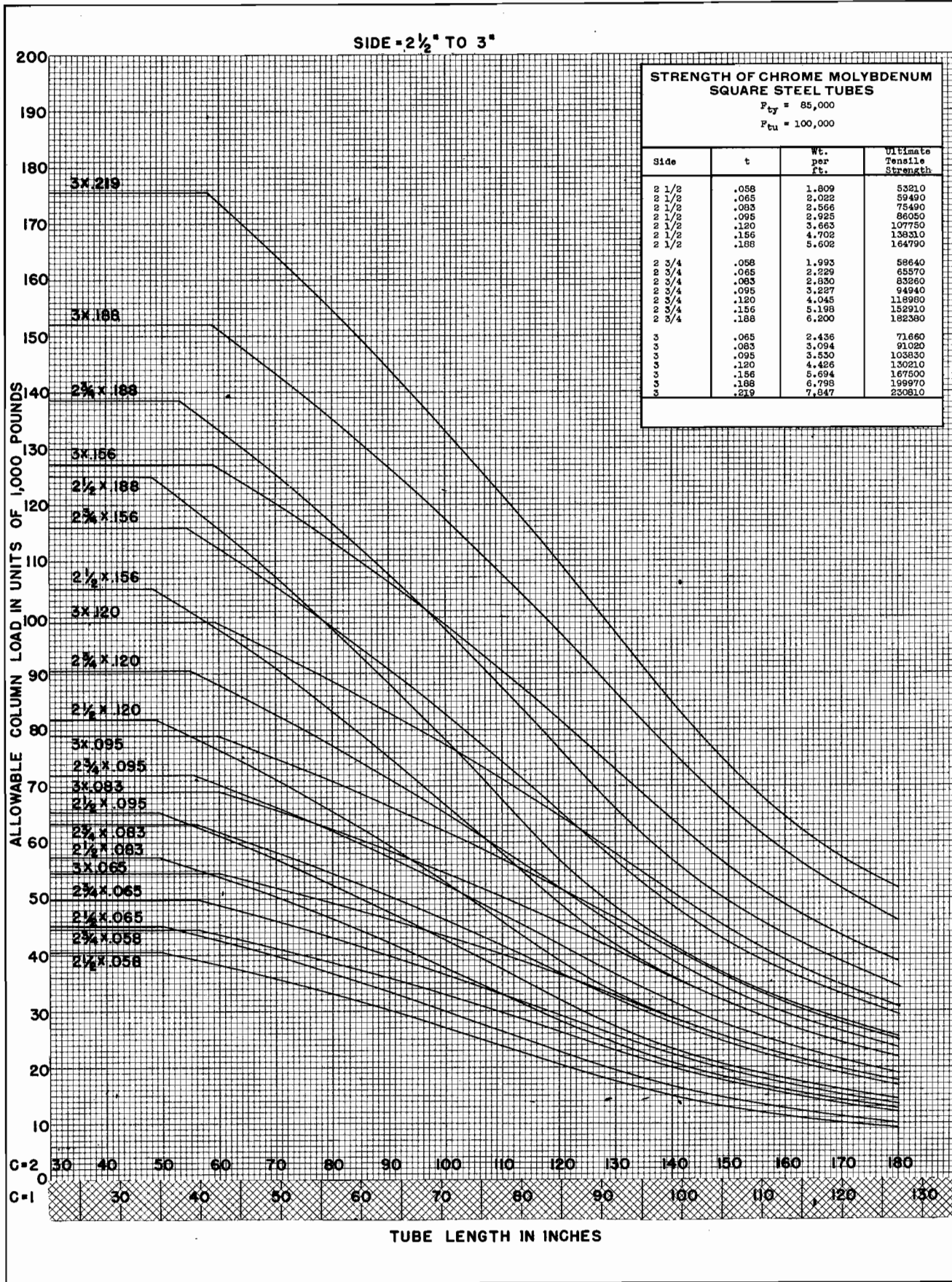




For Squares — 75,000 Yield See III — 37



For Squares — 75,000 Yield See III — 37



For Squares — 75,000 Yield See III — 37